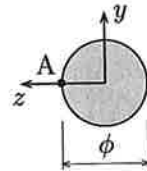


Seção transversal:



Ferro fundido

ASTM A-47

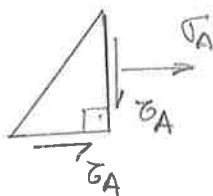
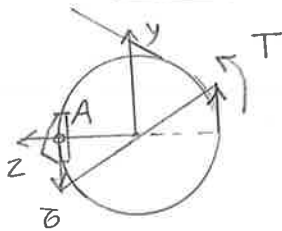
$$f_t = 34 \text{ kN/cm}^2$$

$$E = 62 \text{ kN/cm}^2$$

(Aço S355

$$f_y = 35,5 \text{ kN/cm}^2$$

1. Tensões na seção transversal:

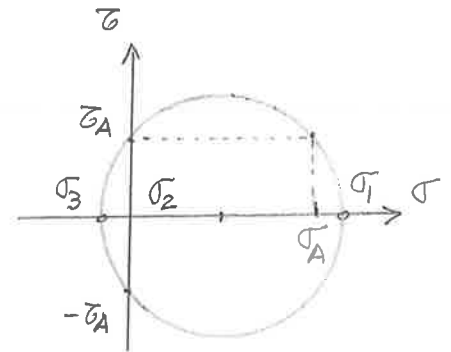


$$A = \pi R^2 = \pi 2^2 = 12,57 \text{ cm}^2$$

$$I_p = \frac{\pi R^4}{2} = \frac{\pi 2^4}{2} = 25,13 \text{ cm}^4$$

$$\sigma_A = \frac{N}{A} = \frac{F}{12,57} = 7,96 \times 10^{-2} F$$

$$\tau_A = \frac{M_T}{I_p} R = \frac{200}{25,13} \times 2 = 15,92$$



2. Tensões principais em A

$$\sigma_{1,3} = \frac{\sigma_A}{2} \pm \sqrt{\left(\frac{\sigma_A}{2}\right)^2 + \tau_A^2} = 3,98 \times 10^{-2} F \pm \sqrt{(3,98 \times 10^{-2} F)^2 + 15,92^2}$$

$$\therefore \begin{cases} \sigma_3 = 3,98 \times 10^{-2} F - \sqrt{15,83 \times 10^{-4} F^2 + 253,3} \\ \sigma_2 = 0 \\ \sigma_1 = 3,98 \times 10^{-2} F + \sqrt{15,83 \times 10^{-4} F^2 + 253,3} \end{cases}$$

$$\begin{aligned} \sigma_{\text{máx}} &= (\sigma_1 - \sigma_3) / 2 \\ &= \sqrt{15,83 \times 10^{-4} F^2 + 253,3} \end{aligned}$$

3. Critérios de resistência

a) Ferro fundido ASTM-47 (material frágil)

- Critério de Rankine $\sigma_{eq} = \sigma_1 < f_t$

$$\sigma_{eq} = 3,98 \times 10^{-2} F + \sqrt{15,83 \times 10^{-4} F^2 + 253,3} = 34 \leftarrow \text{condição limite de ruptura}$$

$$\begin{aligned} 15,83 \times 10^{-4} F^2 + 253,3 &= (34 - 3,98 \times 10^{-2} F)^2 \\ &= 1156 - 2,706 F + 15,83 \times 10^{-4} F^2 \end{aligned}$$

$$2,706 F = 902,7 \Rightarrow F = 333,6 \text{ kN}$$

- Critério de Mohr-Coulomb

$$\sigma_{eq} = \sigma_1 - \frac{\sigma_3}{m} < f_t \quad m = \frac{f_c}{f_t}$$

ou

$$\sigma_{eq} = \left(1 - \frac{1}{m}\right) \frac{\sigma_A}{2} + \left(1 + \frac{1}{m}\right) \sqrt{\left(\frac{\sigma_A}{2}\right)^2 + \tau_A^2}$$

$$m = \frac{62}{34} = 1,824$$

$$\gamma_m = 0,548$$

$$\sigma_{eq} = 0,452 \times 3,98 \times 10^{-2} F + 1,548 \sqrt{15,83 \times 10^{-4} F^2 + 253,3} = 34$$

$$15,83 \times 10^{-4} F^2 + 253,3 = (-1,162 \times 10^{-2} F + 21,96)^2$$

$$= 482,4 - 0,510 F + 1,35 \times 10^{-4} F^2$$

$$14,48 \times 10^{-4} F^2 + 0,510 F - 229,1 = 0 \quad \begin{cases} F = 258,8 \text{ kN} \\ F = -610,6 \text{ kN} \end{cases}$$

b) aço estrutural S355 (dúctil)

- Critério de Tresca

$$\sigma_{eq} = \sigma_1 - \sigma_3 < f_y = 35,5$$

$$\sigma_{eq} = 2 \sqrt{15,83 \times 10^{-4} F^2 + 253,3} = 35,5$$

$$15,83 \times 10^{-4} F^2 + 253,3 = \left(\frac{35,5}{2}\right)^2$$

$$15,83 \times 10^{-4} F^2 = 61,76 \Rightarrow F = 197,5 \text{ kN}$$

Para vigas ($\sigma_2 = 0$):

$$\sigma_{eq} = \sqrt{\sigma_A^2 + 4\tau^2}$$

- Critério de von Mises

$$\sigma_{eq} = \sqrt{\frac{1}{2}[(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2]}$$

$$\sigma_{eq} = \sqrt{\frac{1}{2}[\sigma_1^2 + \sigma_3^2 + (\sigma_3 - \sigma_1)^2]} = \sqrt{\sigma_A^2 + 3\tau_A^2} \quad \left. \begin{array}{l} \text{caso particular} \\ \text{p/ vigas } (\sigma_2 = 0) \end{array} \right\}$$

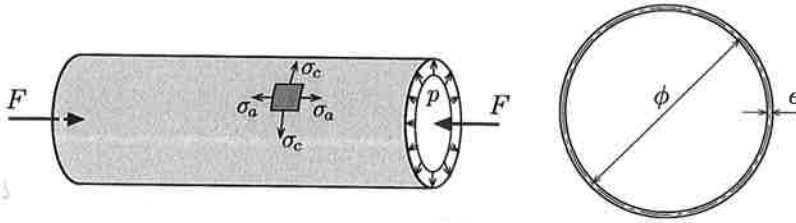
$$(7,96 \times 10^{-2} F)^2 + 3 \times 15,92^2 = 35,5^2$$

$$63,4 \times 10^{-4} F^2 = 499,9 \Rightarrow F = 280,8 \text{ kN}$$

Seção transversal:

$$R = \frac{\phi_m}{2} \approx \frac{\phi_{int}}{2} = 30 \text{ cm}$$

$$A = 2\pi R e = 60\pi e$$



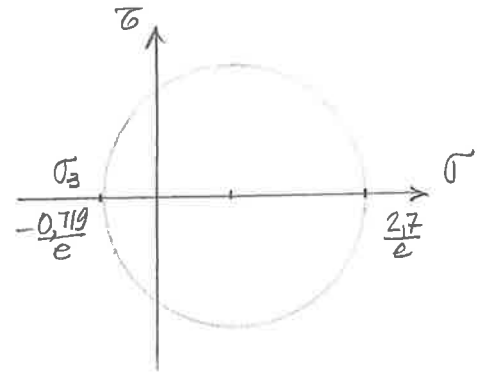
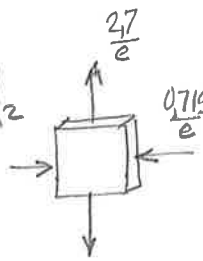
1. Estado de tensão na parede do tubo

$$\sigma_a = \frac{N}{A} + \frac{pR}{2e} = \frac{-390}{60\pi e} + \frac{0,09 \times 30}{2e} = \frac{-2,069}{e} + \frac{1,35}{e} = -\frac{0,719}{e}$$

$$\sigma_b = \frac{pR}{e} = \frac{0,09 \times 30}{e} = \frac{2,7}{e}$$

- Tensões principais (admitir-se que $p < \frac{0,719}{e}$ ou $\sigma_2 = -p$)

$$\begin{cases} \sigma_1 = \frac{2,7}{e} \\ \sigma_2 = -p = -0,09 \frac{\text{MN}}{\text{cm}^2} \\ \sigma_3 = -\frac{0,719}{e} \end{cases}$$



2. Critérios de Resistência

a) ASTM A-48

- Critério de Rankine

$$\sigma_{eq} = \sigma_1 \leq \bar{f}_t$$

$$\sigma_1 = \frac{2,7}{e} = \frac{17}{2,3} \Rightarrow e \leq 0,365 \text{ cm}$$

- Critério de Mohr-Coulomb: $\sigma_{eq} = \sigma_1 - \frac{\sigma_3}{m} \leq \bar{f}_t$ $m = \frac{f_c}{f_t} = \frac{65}{17} = 3,82$

$$\sigma_1 - \frac{\sigma_3}{m} = \frac{2,7}{e} - \frac{(-\frac{0,719}{e})}{3,82} = \frac{17}{2,3} \Rightarrow \frac{2,888}{e} = \frac{17}{2,3} \Rightarrow e \leq 0,390 \text{ cm}$$

b) ASTM A-36

- Critério de Tresca: $\sigma_{eq} = \sigma_1 - \sigma_3 = \bar{f}_y$

$$\sigma_1 - \sigma_3 = \frac{2,7}{e} + \frac{0,719}{e} = \frac{25}{2,3} \Rightarrow e \geq 0,315 \text{ cm}$$

- Critério de von Mises: $\sigma_{eq} = \sqrt{\frac{1}{2}[(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2]} = \bar{f}_y$

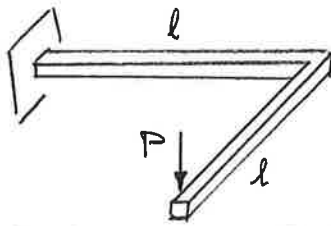
$$\left(\frac{2,7}{e} + 0,09\right)^2 + \left(-0,09 + \frac{0,719}{e}\right)^2 + \left(-\frac{0,719}{e} - \frac{2,7}{e}\right)^2 = 2 \left(\frac{25}{2,3}\right)^2$$

resolvendo: $e = 0,288 \text{ cm}$

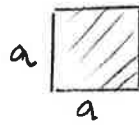
Obs. considerando $\sigma_2 = 0$ obtemos $e = 0,287 \text{ cm}$.

Ex.

[Mário] Determine o máximo valor de P usando diferentes critérios de resistência. Dadas: $\sigma_{rt} = 15 \frac{\text{kN}}{\text{cm}^2}$, $\sigma_{rc} = 33 \frac{\text{kN}}{\text{cm}^2}$, $\gamma = 3$, $l = 30 \text{ cm}$ e $a = 3 \text{ cm}$.

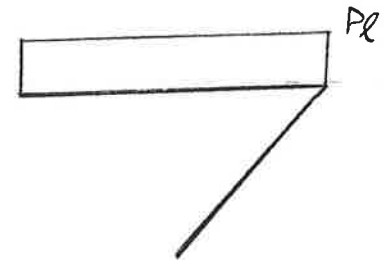
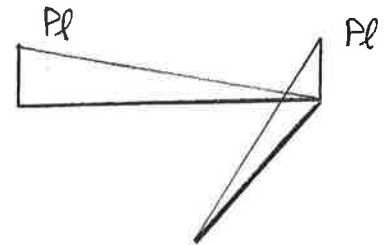


ST:



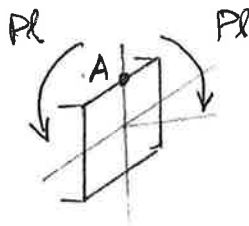
$$Pl = 30P \text{ (kNm)}$$

(M)



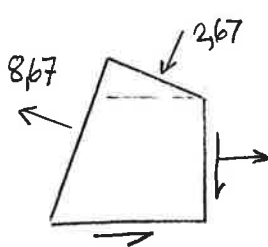
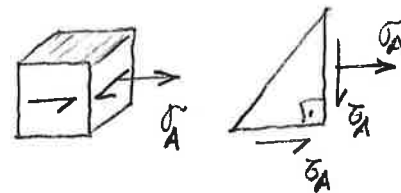
(T)

• a ST de engaste é a mais solicitada



$$\sigma_A = \frac{M}{W} = \frac{Pl}{a^3/6} = \frac{6Pl}{a^3}$$

$$\tau_A = \frac{M_T}{W_T} = \frac{Pl}{320a^3} = 4,81 \frac{Pl}{a^3}$$



$$\sigma_{I,II} = \frac{\sigma_A}{2} \pm \sqrt{\left(\frac{\sigma_A}{2}\right)^2 + \tau_A^2} = \left(3 \pm \sqrt{9 + 4,81^2}\right) \frac{Pl}{a^3} = (3 \pm 5,67) \frac{Pl}{a^3}$$

$$\tan \alpha_1 = \frac{\sigma_1 - \sigma_2}{2\tau_{\text{máx}}} = \frac{8,67 - 6}{4,81} = 0,555 \Rightarrow \alpha_1 = 29^\circ \quad \left| \quad \tau_{\text{máx}} = 5,67 \frac{Pl}{a^3} \right.$$

$$\text{Estado Tensão:} \quad \sigma_3 = -2,67 \frac{Pl}{a^3}; \quad \sigma_2 = 0; \quad \sigma_1 = 8,67 \frac{Pl}{a^3}$$

a) Critério da máx. tensão normal (Rankine) $\sigma_{eq} = \sigma_1 \leq \sigma_E$

$$8,67 \cdot \frac{Pl}{a^3} \leq \frac{\sigma_{rt}}{3} \Rightarrow P \leq \frac{15 \times 3^3}{3 \times 8,67 \times 30} = 0,52 \text{ kN}$$

b) Critério da máx. deformação linear ($\nu = 0,25$) $\sigma_{eq} = \sigma_1 - \nu(\sigma_2 + \sigma_3) \leq \sigma_E$

$$\left[8,67 - 0,25(0 - 2,67)\right] \frac{Pl}{a^3} \leq \frac{\sigma_{rt}}{3} \Rightarrow P \leq \frac{15 \times 3^3}{3 \times 9,34 \times 30} = 0,48 \text{ kN}$$

c) Critério de Mohr-Coulomb $\sigma_{eq} = \sigma_1 - \frac{\sigma_3}{m} \leq \sigma_E \quad m = \frac{|\sigma_c|}{\sigma_t}$

$$\left[8,67 - \frac{15}{33} \times (-2,67)\right] \frac{Pl}{a^3} \leq \frac{\sigma_{rt}}{3} \Rightarrow P \leq \frac{15 \times 3^3}{3 \times 9,88 \times 30} = 0,46 \text{ kN}$$

Ferro fundido

Barra de aço: $\sigma_y = 21 \frac{\text{kN}}{\text{cm}^2}$ $\gamma = 2$

d) critério de Tresca

$$\sigma_{eq} = \sigma_1 - \sigma_3 \leq \sigma_y$$

$$[8,67 - (-2,67)] \frac{Pl}{a^3} \leq \frac{\sigma_y}{2} \Rightarrow P \leq \frac{21}{2} \times \frac{3^3}{11,34 \times 30} = \underline{9,83 \text{ kN}}$$

e) critério de von Mises

$$\sigma_{eq} = \left\{ \frac{1}{2} [(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_3 - \sigma_1)^2] \right\}^{1/2} \leq \sigma_e$$

$$\left\{ \frac{1}{2} [8,67^2 + 2,67^2 + 11,34^2] \right\}^{1/2} \frac{Pl}{a^3} \leq \frac{21}{2} \Rightarrow P \leq \frac{21}{2} \times \frac{3^3}{10,27 \times 30} = \underline{9,92 \text{ kN}}$$