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Discrete Systems:
ARGYRIS' NATURAL MEMBRANE ELEMENT

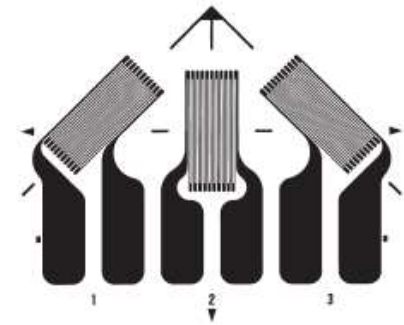
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Argyris' Natural Membrane Element

Argyris ~1974

A membrane finite element based on natural deformations (measured along the sides of the element), able to cope with large displacements and large deformations.

- Akin to a “strain rosette” plane stress finite element:



Meek ~1991

- A corrotational description;
- Small strains.

Pauletti ~2003

- a more concise notation;
- distinction between the constitutive and geometric parts of the element tangent stiffness;
- the “simplest possible membrane finite element”;
- large displacements / small strains (a few percent...)

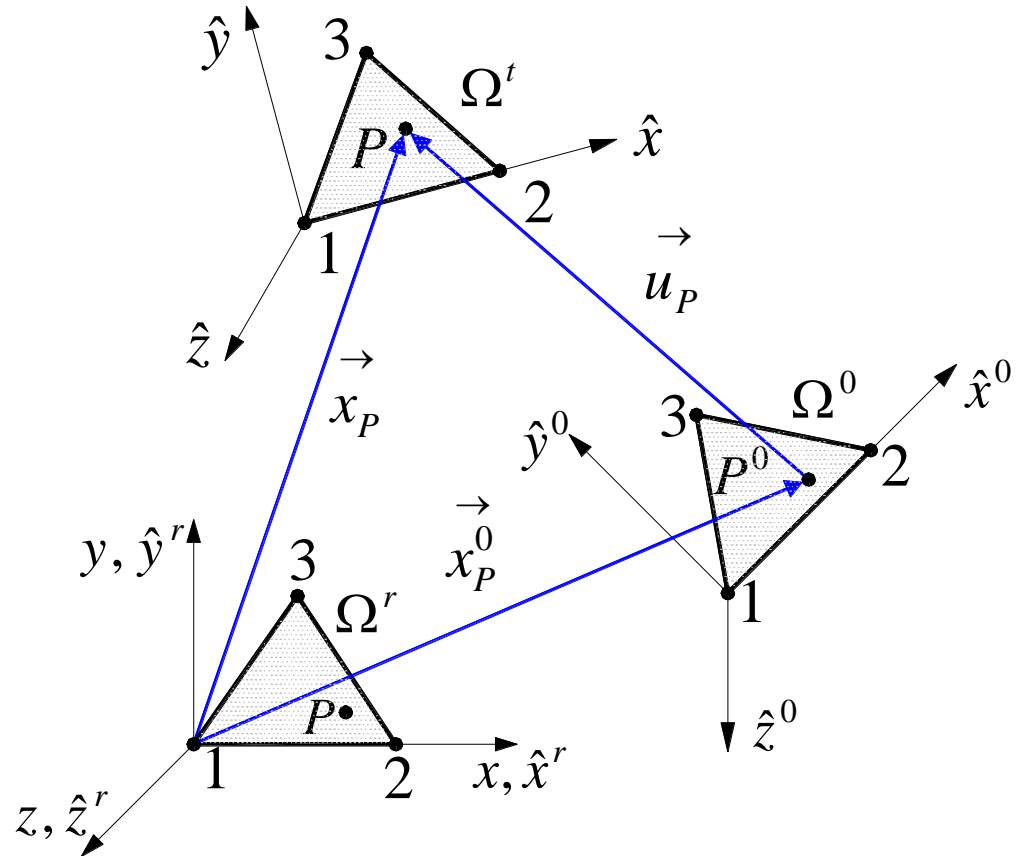
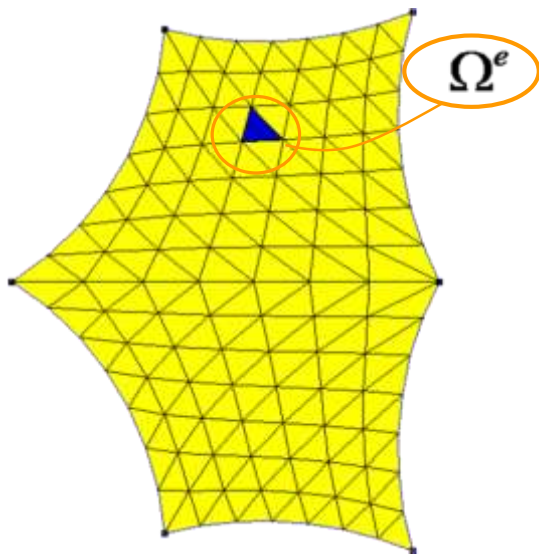
Pauletti (2006)

- first publication on the natural force density concept

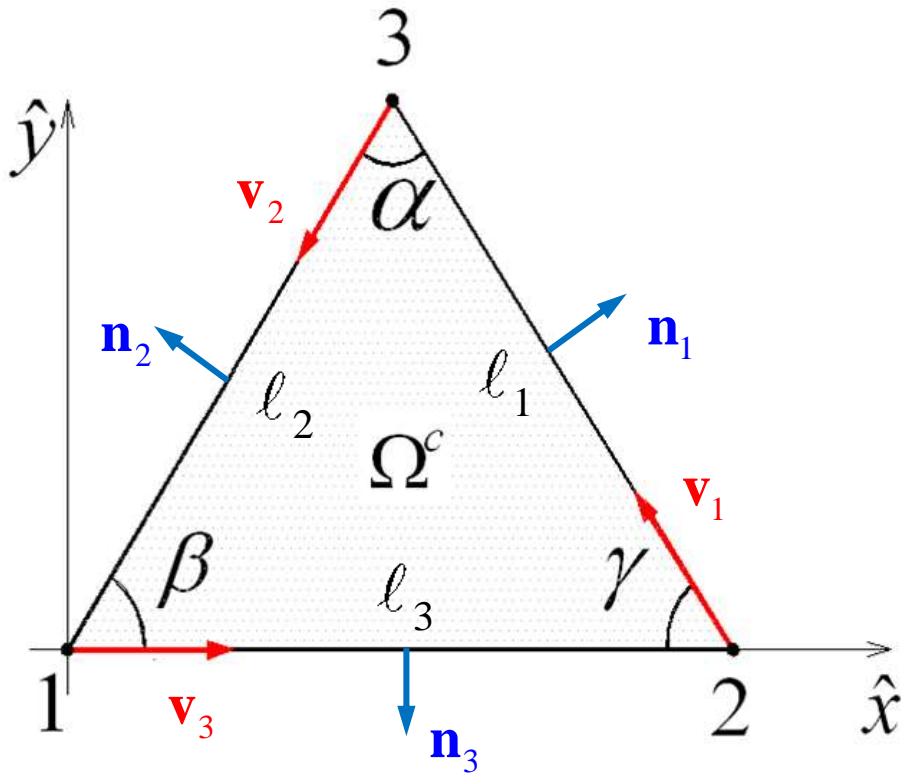
R.M.O. Pauletti, “An extension of the force density procedure to membrane structures”

IASS Symposium / APCS Conference – New Olympics, New Shell and Spatial Structures, Beijing, 2006

Reference, Initial and Current Configurations For Argyris Element



Element Description



$$\mathbf{x}_{0,c} = \begin{bmatrix} \mathbf{x}_1 \\ \mathbf{x}_2 \\ \mathbf{x}_3 \end{bmatrix}_{0,c} \quad \mathbf{u} = \mathbf{x}_c - \mathbf{x}_0 = \begin{bmatrix} \mathbf{u}_1 \\ \mathbf{u}_2 \\ \mathbf{u}_3 \end{bmatrix}$$

Side lengths:

$$l_i = \|\mathbf{x}_k - \mathbf{x}_j\| \quad \{i, j, k\} \equiv \{1, 2, 3\}$$

(in cyclic permutation)

Unit side vectors:
$$\mathbf{v}_i = \frac{\mathbf{x}_k^e - \mathbf{x}_j^e}{l_i}$$

Unit normal vectors:
$$\mathbf{n}_i = -\hat{\mathbf{k}} \times \mathbf{v}_i$$

Element Stress Field and Vector of Internal Nodal Forces

Cauchy Plane Stress Tensor:

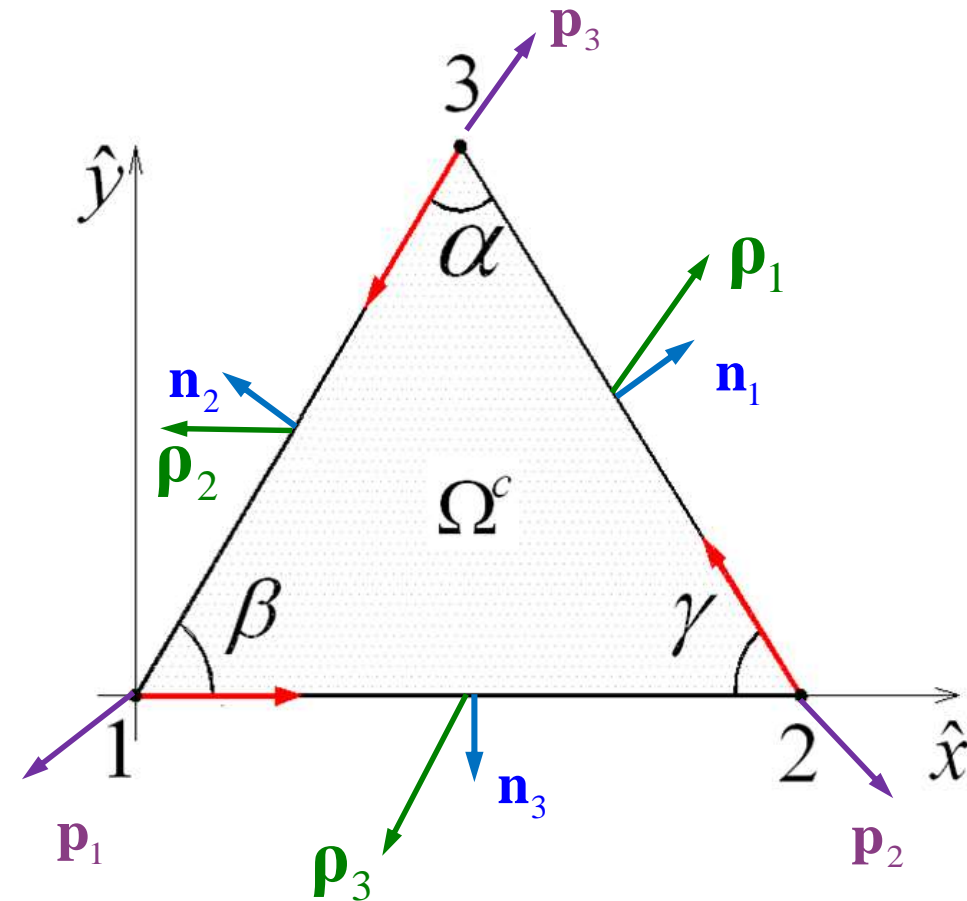
$$\hat{\boldsymbol{\sigma}} = \begin{bmatrix} \sigma_{\hat{x}} & \tau_{\hat{x}\hat{y}} & 0 \\ \tau_{\hat{x}\hat{y}} & \sigma_{\hat{y}} & 0 \\ 0 & 0 & 0 \end{bmatrix} \equiv \begin{bmatrix} \sigma_{\hat{x}} \\ \sigma_{\hat{y}} \\ \tau_{\hat{x}\hat{y}} \end{bmatrix}$$

Side stress vectors:

$$\boldsymbol{\rho}_i = \hat{\boldsymbol{\sigma}} \mathbf{n}_i$$

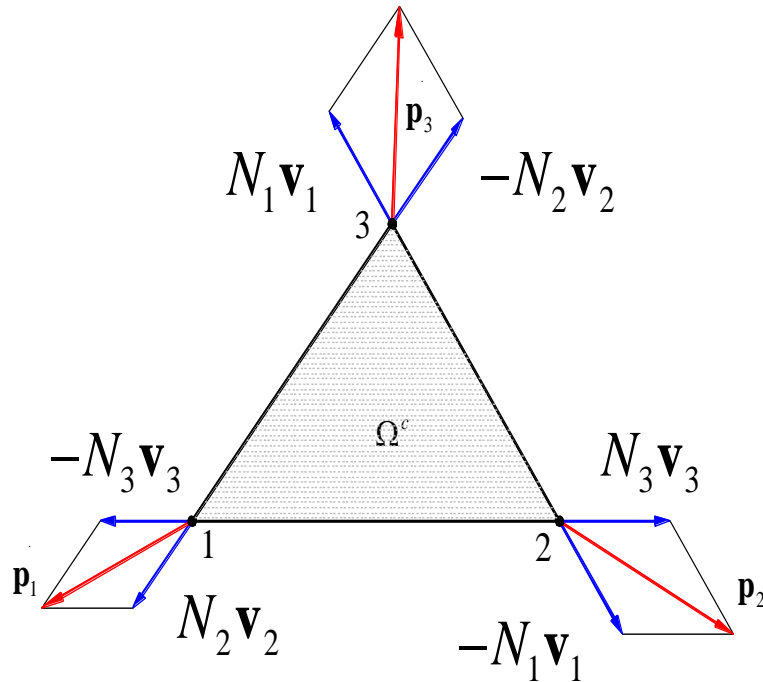
Vector of internal nodal forces:

$$\mathbf{p}^e = \begin{bmatrix} \mathbf{p}_1^e \\ \mathbf{p}_2^e \\ \mathbf{p}_3^e \end{bmatrix} = \frac{t}{2} \begin{bmatrix} l_2 \boldsymbol{\rho}_2 + l_3 \boldsymbol{\rho}_3 \\ l_1 \boldsymbol{\rho}_1 + l_3 \boldsymbol{\rho}_3 \\ l_1 \boldsymbol{\rho}_1 + l_2 \boldsymbol{\rho}_2 \end{bmatrix}$$



Vector of Natural Forces

The vector of internal forces can be decomposed into components parallel to the element sides:



$$\mathbf{p}^e = \begin{bmatrix} N_2 \mathbf{v}_2^e - N_3 \mathbf{v}_3^e \\ N_3 \mathbf{v}_3^e - N_1 \mathbf{v}_1^e \\ N_1 \mathbf{v}_1^e - N_2 \mathbf{v}_2^e \end{bmatrix}$$

Vector of Natural Forces

$$\mathbf{N} = \begin{bmatrix} N_1 \\ N_2 \\ N_3 \end{bmatrix}$$

Natural Stresses

Comparing both expressions available for p^e :

$$\begin{bmatrix} N_2 \mathbf{v}_2^e - N_3 \mathbf{v}_3^e \\ N_3 \mathbf{v}_3^e - N_1 \mathbf{v}_1^e \\ N_1 \mathbf{v}_1^e - N_2 \mathbf{v}_2^e \end{bmatrix} = \frac{t}{2} \begin{bmatrix} l_2 \boldsymbol{\rho}_2 + l_3 \boldsymbol{\rho}_3 \\ l_1 \boldsymbol{\rho}_1 + l_3 \boldsymbol{\rho}_3 \\ l_1 \boldsymbol{\rho}_1 + l_2 \boldsymbol{\rho}_2 \end{bmatrix}$$

We obtain the Vector of Natural Forces, as function of Cauchy Stresses, and we identify some “Natural Stresses” ($\sigma_1, \sigma_2, \sigma_3$):

$$\mathbf{N} = \begin{bmatrix} N_1 \\ N_2 \\ N_3 \end{bmatrix} = \frac{t}{2} \begin{bmatrix} h_1 \left(\frac{\cos \beta}{\sin \gamma \sin \alpha} \sigma_{\hat{y}} - \frac{\sin \beta}{\sin \gamma \sin \alpha} \tau_{\hat{x}\hat{y}} \right) \\ h_2 \left(\frac{\cos \gamma}{\sin \beta \sin \alpha} \sigma_{\hat{y}} + \frac{\sin \gamma}{\sin \beta \sin \alpha} \tau_{\hat{x}\hat{y}} \right) \\ h_3 \left(\sigma_{\hat{x}} - \frac{\cos \beta \cos \gamma}{\sin \beta \sin \gamma} \sigma_{\hat{y}} + \frac{\cos(\beta - \gamma)}{\sin \beta \sin \gamma} \tau_{\hat{x}\hat{y}} \right) \end{bmatrix}$$

$$\mathbf{N} = \begin{bmatrix} N_1 \\ N_2 \\ N_3 \end{bmatrix} = \frac{t}{2} \begin{bmatrix} h_1 \sigma_1 \\ h_2 \sigma_2 \\ h_3 \sigma_3 \end{bmatrix}$$

Vector of Natural Stresses

We group the Natural Stresses'' ($\sigma_1, \sigma_2, \sigma_3$) in a Vector of Natural Stresses:

$$\boldsymbol{\sigma}_n = \begin{bmatrix} \sigma_1 \\ \sigma_2 \\ \sigma_3 \end{bmatrix} = \begin{bmatrix} 0 & \frac{\cos \beta}{\sin \gamma \sin \alpha} & -\frac{\sin \beta}{\sin \gamma \sin \alpha} \\ 0 & \frac{\cos \gamma}{\sin \beta \sin \alpha} & \frac{\sin \gamma}{\sin \beta \sin \alpha} \\ 1 & -\frac{\cos \beta \cos \gamma}{\sin \beta \sin \gamma} & \frac{\sin(\beta - \gamma)}{\sin \beta \sin \gamma} \end{bmatrix} \begin{bmatrix} \sigma_{\hat{x}} \\ \sigma_{\hat{y}} \\ \tau_{\hat{x}\hat{y}} \end{bmatrix}$$

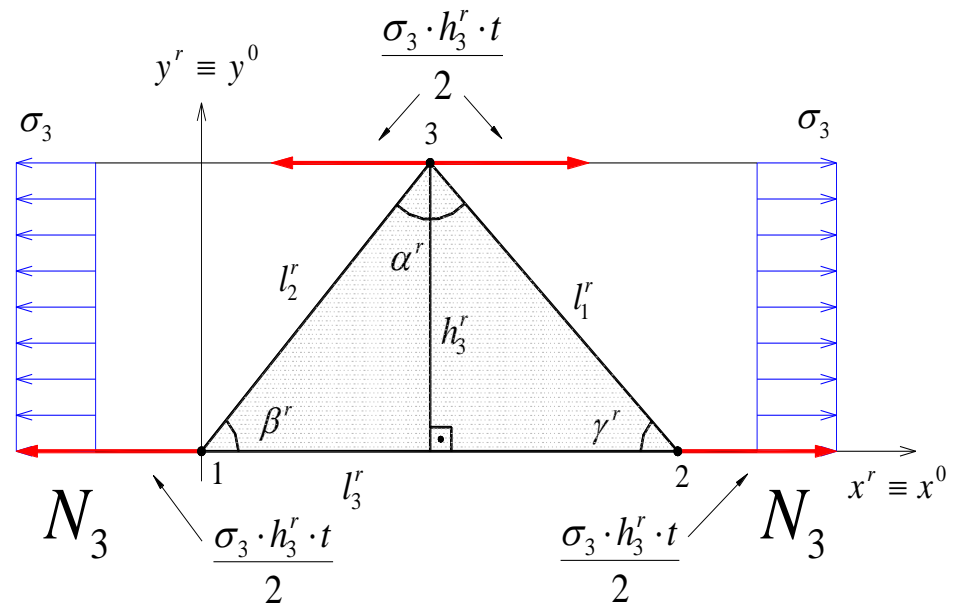
$$\boldsymbol{\sigma}_n = \mathbf{T}^{-T} \hat{\boldsymbol{\sigma}}$$

Exercise 11. Verify the above expression!

Vector of Natural Stresses

Each natural force N_i can be understood as the nodal resultant of each natural normal stress field σ_i

$$N_i = \frac{t}{2} h_i \sigma_i$$



And since $h_i = \frac{2A}{l_i}$ ∴

$$N_i = V \frac{\sigma_i}{l_i}$$

Relationship between the Vectors of Natural Forces and Stresses

In matrix form:

$$\mathbf{N} = V \mathcal{L}^{-1} \boldsymbol{\sigma}_n$$

Length matrix:

$$\mathcal{L} = \begin{bmatrix} l_1 & 0 & 0 \\ 0 & l_2 & 0 \\ 0 & 0 & l_3 \end{bmatrix}$$

$$\boldsymbol{\sigma}_n = \begin{bmatrix} \sigma_1 \\ \sigma_2 \\ \sigma_3 \end{bmatrix}$$

Vector of Natural Stresses

Vector of Natural Deformations

The deformations along the sides of the element are collected in a 'Vector of Natural Deformations':

$$\begin{bmatrix} \varepsilon_1 \\ \varepsilon_2 \\ \varepsilon_3 \end{bmatrix} = \begin{bmatrix} \cos^2 \gamma & \sin^2 \gamma & -\sin \gamma \cos \gamma \\ \cos^2 \beta & \sin^2 \beta & -\sin \beta \cos \beta \\ 1 & 0 & 0 \end{bmatrix} \begin{bmatrix} \varepsilon_{\hat{x}} \\ \varepsilon_{\hat{y}} \\ \gamma_{\hat{x}\hat{y}} \end{bmatrix}$$

$$\boldsymbol{\varepsilon}_n = \mathbf{T} \hat{\boldsymbol{\varepsilon}}$$

Linearized
Green
Strains

Exercise 12. Verify the above expression!

We remark that $\boldsymbol{\sigma}_n$ and $\boldsymbol{\varepsilon}_n$ are energetically conjugate.

Indeed, by the Principle of Virtual Work:

$$\delta \hat{\boldsymbol{\varepsilon}}^T \hat{\boldsymbol{\sigma}} = \delta \boldsymbol{\varepsilon}_n^T \boldsymbol{\sigma}_n, \quad \forall \delta \hat{\boldsymbol{\varepsilon}}$$

$$\delta \hat{\boldsymbol{\varepsilon}}^T \hat{\boldsymbol{\sigma}} = (\mathbf{T} \delta \hat{\boldsymbol{\varepsilon}})^T \boldsymbol{\sigma}_n = \delta \hat{\boldsymbol{\varepsilon}}^T \mathbf{T}^T \boldsymbol{\sigma}_n, \quad \forall \delta \hat{\boldsymbol{\varepsilon}}$$

Thus: $\boldsymbol{\sigma}_n = \mathbf{T}^{-T} \hat{\boldsymbol{\sigma}}$, as deduced before.

Tangent Stiffness Matrix for Argyris' Element

$$\mathbf{k}_t = \frac{\partial \mathbf{g}}{\partial \mathbf{u}} = \mathbf{N}^T \frac{\partial \mathbf{C}}{\partial \mathbf{u}} + \mathbf{C} \frac{\partial \mathbf{N}}{\partial \mathbf{u}} - \frac{\partial \mathbf{f}}{\partial \mathbf{u}}$$

Geometric Stiffness Matrix

$$\mathbf{k}_g = \mathbf{N}^T \frac{\partial \mathbf{C}}{\partial \mathbf{u}} = \begin{bmatrix} N_2 \frac{\partial \mathbf{v}_2}{\partial \mathbf{u}} - N_3 \frac{\partial \mathbf{v}_3}{\partial \mathbf{u}} \\ -N_1 \frac{\partial \mathbf{v}_1}{\partial \mathbf{u}} + N_3 \frac{\partial \mathbf{v}_3}{\partial \mathbf{u}} \\ N_1 \frac{\partial \mathbf{v}_1}{\partial \mathbf{u}} - N_2 \frac{\partial \mathbf{v}_2}{\partial \mathbf{u}} \end{bmatrix}$$

$$\mathbf{k}_g = \begin{bmatrix} \frac{N_2}{l_2} (\mathbf{I}_3 - \mathbf{v}_2 \mathbf{v}_2^T) + \frac{N_3}{l_3} (\mathbf{I}_3 - \mathbf{v}_3 \mathbf{v}_3^T) & -\frac{N_3}{l_3} (\mathbf{I}_3 - \mathbf{v}_3 \mathbf{v}_3^T) & -\frac{N_2}{l_2} (\mathbf{I}_3 - \mathbf{v}_2 \mathbf{v}_2^T) \\ -\frac{N_3}{l_3} (\mathbf{I}_3 - \mathbf{v}_3 \mathbf{v}_3^T) & \frac{N_1}{l_1} (\mathbf{I}_3 - \mathbf{v}_1 \mathbf{v}_1^T) + \frac{N_3}{l_3} (\mathbf{I}_3 - \mathbf{v}_3 \mathbf{v}_3^T) & -\frac{N_1}{l_1} (\mathbf{I}_3 - \mathbf{v}_1 \mathbf{v}_1^T) \\ -\frac{N_2}{l_2} (\mathbf{I}_3 - \mathbf{v}_2 \mathbf{v}_2^T) & -\frac{N_1}{l_1} (\mathbf{I}_3 - \mathbf{v}_1 \mathbf{v}_1^T) & \frac{N_1}{l_1} (\mathbf{I}_3 - \mathbf{v}_1 \mathbf{v}_1^T) + \frac{N_2}{l_2} (\mathbf{I}_3 - \mathbf{v}_2 \mathbf{v}_2^T) \end{bmatrix}$$

Exact!

Tangent Stiffness Matrix for Argyris' Element

$$\mathbf{k}_t = \frac{\partial \mathbf{g}}{\partial \mathbf{u}} = \mathbf{N}^T \frac{\partial \mathbf{C}}{\partial \mathbf{u}} + \mathbf{C} \frac{\partial \mathbf{N}}{\partial \mathbf{u}} - \frac{\partial \mathbf{f}}{\partial \mathbf{u}}$$

External Stiffness Matrix

$$\mathbf{k}_{\text{ext}} = \frac{\partial \mathbf{f}}{\partial \mathbf{u}}$$

External force vector:

$$\mathbf{f} = \mathbf{f}_{\text{weight}} - \mathbf{f}_{\text{wind}} = \frac{V\rho}{3} \begin{bmatrix} \mathbf{I}_3 \\ \mathbf{I}_3 \\ \mathbf{I}_3 \end{bmatrix} \mathbf{g} - \frac{pA}{3} \begin{bmatrix} \mathbf{I}_3 \\ \mathbf{I}_3 \\ \mathbf{I}_3 \end{bmatrix} \mathbf{n}$$

$$\mathbf{k}_{\text{ext}} = \frac{\partial \mathbf{f}_{\text{wind}}}{\partial \mathbf{u}} = \dots = \frac{p}{6} \begin{bmatrix} \Lambda_1 & \Lambda_2 & \Lambda_3 \\ \Lambda_1 & \Lambda_2 & \Lambda_3 \\ \Lambda_1 & \Lambda_2 & \Lambda_3 \end{bmatrix}$$

$$\Lambda_i = \text{Skew}(\mathbf{I}_i) = \ell_i \begin{bmatrix} 0 & -v_i^z & v_i^y \\ v_i^z & 0 & -v_i^x \\ -v_i^y & v_i^x & 0 \end{bmatrix}, i = 1, 2, 3$$

Exact!

Exercise 13. Verify the above expression for \mathbf{k}_{ext} !

Tangent Stiffness Matrix for Argyris' Element

$$\mathbf{k}_t = \frac{\partial \mathbf{g}}{\partial \mathbf{u}} = \mathbf{N}^T \frac{\partial \mathbf{C}}{\partial \mathbf{u}} + \mathbf{C} \frac{\partial \mathbf{N}}{\partial \mathbf{u}} - \frac{\partial \mathbf{f}}{\partial \mathbf{u}}$$

Constitutive Stiffness Matrix

$$\mathbf{k}_c = \mathbf{C} \frac{\partial \mathbf{N}}{\partial \mathbf{u}} \quad \textit{Exact!}$$

Defining the vector of Natural Displacements $\mathbf{a} = \begin{bmatrix} l_1 - l_1^0 \\ l_2 - l_2^0 \\ l_3 - l_3^0 \end{bmatrix}$

There exist some kind of relationship $\mathbf{N} = \mathbf{N}(\mathbf{a})$ so that

$$\mathbf{k}_c = \mathbf{C} \frac{\partial \mathbf{N}}{\partial \mathbf{a}} \frac{\partial \mathbf{a}}{\partial \mathbf{u}} = \mathbf{C} \mathbf{k}_n \mathbf{C}^T$$

$$\mathbf{k}_n = \frac{\partial \mathbf{N}}{\partial \mathbf{a}} \quad \textit{'is the Natural Stiffness Matrix'}$$

Tangent Stiffness Matrix for Argyris' Element

A simplification: Linear elastic material behavior

Thus, a linear relationship $\mathbf{N} = \mathbf{k}_n^r \mathbf{a}$ exists

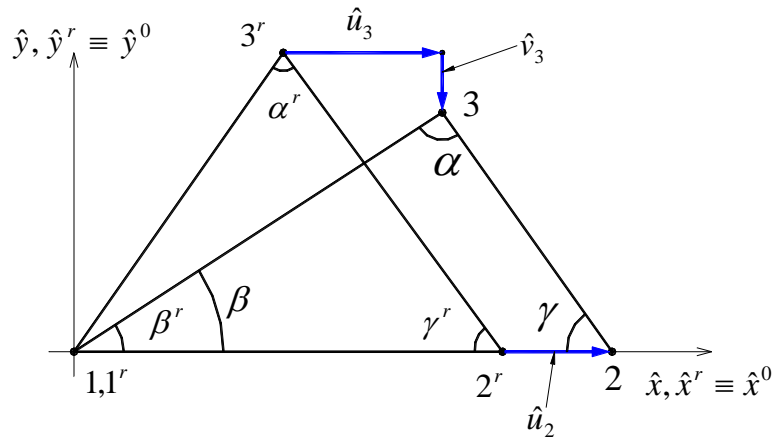
Where $\mathbf{k}_n^r = \frac{\partial \mathbf{N}}{\partial \mathbf{a}}$ is a 3x3 constant natural stiffness matrix

And therefore

$$\mathbf{k}_c = \mathbf{C} \mathbf{k}_n^r \mathbf{C}^T$$

?!

A linear elastic simplification for K_c



$$\hat{\boldsymbol{\varepsilon}} = \begin{bmatrix} \varepsilon_{\hat{x}} \\ \varepsilon_{\hat{y}} \\ \gamma_{\hat{x}\hat{y}} \end{bmatrix} = \begin{bmatrix} \frac{\partial \hat{u}}{\partial \hat{x}} \\ \frac{\partial \hat{v}}{\partial \hat{y}} \\ \frac{\partial \hat{v}}{\partial \hat{x}} + \frac{\partial \hat{u}}{\partial \hat{y}} \end{bmatrix} = \begin{bmatrix} \frac{u_2}{x_2^r} \\ \frac{v_3}{y_3^r} \\ \frac{x_2^r u_3 - x_3^r u_2}{x_2^r y_3^r} \end{bmatrix}$$

$$\boldsymbol{\varepsilon}_n = \begin{bmatrix} \varepsilon_1 \\ \varepsilon_2 \\ \varepsilon_3 \end{bmatrix} = \begin{bmatrix} \frac{\Delta l_1}{l_1^r} \\ \frac{\Delta l_2}{l_2^r} \\ \frac{\Delta l_3}{l_3^r} \end{bmatrix} = \begin{bmatrix} \frac{1}{l_1^r} & 0 & 0 \\ 0 & \frac{1}{l_2^r} & 0 \\ 0 & 0 & \frac{1}{l_3^r} \end{bmatrix} \begin{bmatrix} \Delta l_1 \\ \Delta l_2 \\ \Delta l_3 \end{bmatrix} = \mathbf{L}_r^{-1} \mathbf{a}$$

$$\boldsymbol{\varepsilon}_n = \mathbf{T}_r \hat{\boldsymbol{\varepsilon}}$$

$$\mathbf{T}_r = \begin{bmatrix} \cos^2 \gamma_r & \sin^2 \gamma_r & -\sin \gamma_r \cos \gamma_r \\ \cos^2 \beta_r & \sin^2 \beta_r & -\sin \beta_r \cos \beta_r \\ 1 & 0 & 0 \end{bmatrix}$$

A linear elastic simplification for K_c

Hooke's Law:
$$\hat{\boldsymbol{\sigma}} = \begin{bmatrix} \sigma_{\hat{x}} \\ \sigma_{\hat{y}} \\ \tau_{\hat{x}\hat{y}} \end{bmatrix} = \hat{\mathbf{D}}\hat{\boldsymbol{\varepsilon}} + \hat{\boldsymbol{\sigma}}_0 \quad \hat{\mathbf{D}} = \frac{E}{1-\nu^2} \begin{bmatrix} 1 & \nu & 0 \\ \nu & 1 & 0 \\ 0 & 0 & \frac{1-\nu}{2} \end{bmatrix}$$

But, now:
$$\boldsymbol{\sigma}_n = \mathbf{T}_r^{-T} \hat{\boldsymbol{\sigma}} = \mathbf{T}_r^{-T} \left(\hat{\mathbf{D}}\hat{\boldsymbol{\varepsilon}} + \hat{\boldsymbol{\sigma}}_0 \right) = \mathbf{T}_r^{-T} \hat{\mathbf{D}}\hat{\boldsymbol{\varepsilon}} + \mathbf{T}_r^{-T} \hat{\boldsymbol{\sigma}}_0$$

$$\boldsymbol{\sigma}_n = \mathbf{T}_r^{-T} \hat{\mathbf{D}} \mathbf{T}_r^{-1} \boldsymbol{\varepsilon}_n + \mathbf{T}_r^{-T} \hat{\boldsymbol{\sigma}}_0$$

That is
$$\boldsymbol{\sigma}_n = \mathbf{D}_n \boldsymbol{\varepsilon}_n + \boldsymbol{\sigma}_{n0}$$

Where
$$\mathbf{D}_n = \mathbf{T}_r^{-T} \hat{\mathbf{D}} \mathbf{T}_r^{-1}$$

A linear elastic simplification for K_c

Recalling the Natural Forces: $\mathbf{N} = V^r \mathcal{L}_r^{-1} \boldsymbol{\sigma}_n$

$$\mathbf{N} = V^r \mathcal{L}_r^{-1} \mathbf{D}_n \boldsymbol{\varepsilon}_n = V^r \mathcal{L}_r^{-1} \left(\mathbf{T}_r^{-T} \hat{\mathbf{D}} \mathbf{T}_r^{-1} \right) \mathcal{L}_r^{-1} \mathbf{a}$$

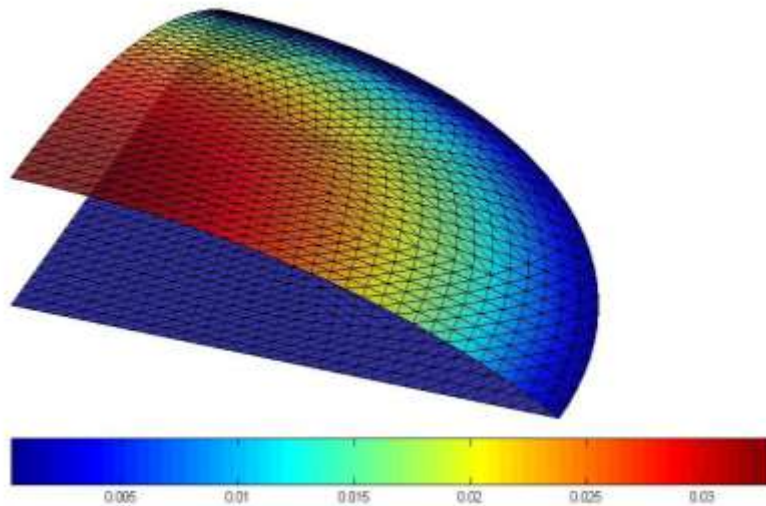
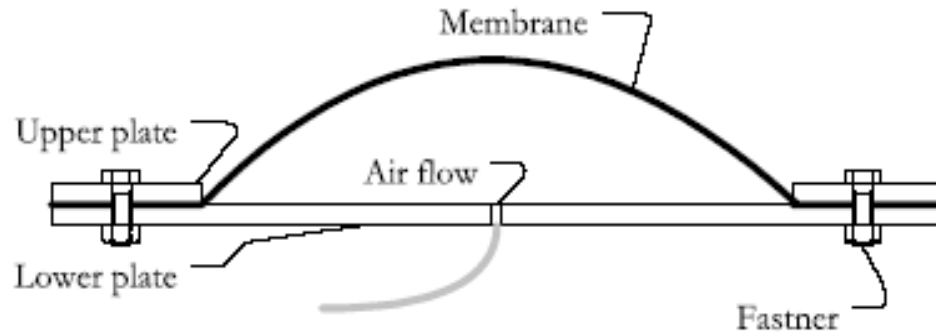
And we arrive to the Natural Stiffness Matrix, (considering small deformations):

$$\mathbf{k}_n^r = \frac{\partial \mathbf{N}}{\partial \mathbf{a}} = V^r \mathcal{L}_r^{-1} \left(\mathbf{T}_r^{-T} \hat{\mathbf{D}} \mathbf{T}_r^{-1} \right) \mathcal{L}_r^{-1}$$

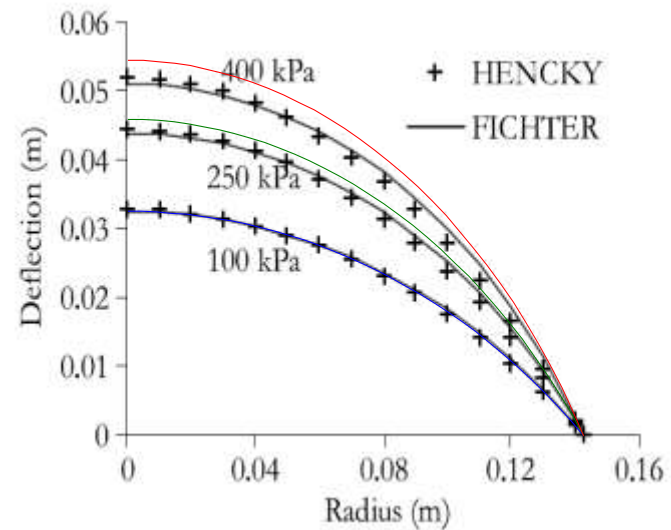
An order 3, symmetric matrix, that can be calculated and stored at the start, and rotated at each Newton's iteration, according to the co-rotational element coordinate system:

$$\mathbf{k}_c = \mathbf{C} \mathbf{k}_n^r \mathbf{C}^T$$

A benchmark: an axisymmetric pressurized membrane



deformed shape,
as calculated by SATS



comparison between SATS
and theoretical results