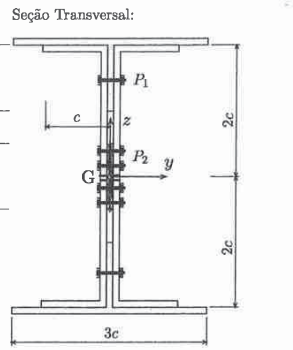
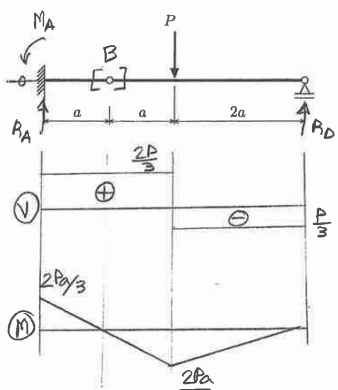


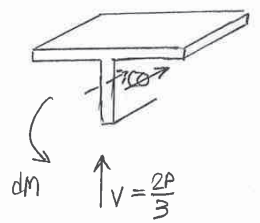
05PRQ3 Ex. 1

$\sum \square \circlearrowleft M_B^{dir} = 0$
 $\{ R_D \times 3a - Pa = 0 \Rightarrow R_D = \frac{P}{3}$
 $\uparrow \{ R_A = \frac{2P}{3}$
 $\sum \square \circlearrowleft M_B^{eq} = 0$
 $\{ -M_A + \frac{2P}{3}a = 0 \Rightarrow M_A = \frac{2Pa}{3}$
 verif.
 $\sum D \{ -\frac{2Pa}{3} + \frac{2P}{3} \times \frac{1}{2}a - P \times 2a = 0$



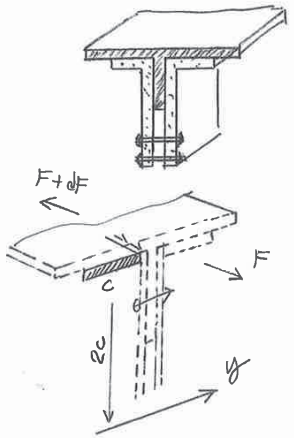
$P = 150 \text{ kN}$ $\delta = 0,5 \text{ cm}$
 $a = 200 \text{ cm}$ $I_y = 56 \delta c^3$
 $c = 10 \text{ cm}$ $\bar{\sigma} = 7,5 \frac{\text{kN}}{\text{cm}^2}$
 $a_1 = a_2 = 10 \text{ cm}$

Parafuso P1



$\bar{S}_1 = (3c\delta) \times 2c + c\delta \times \frac{3c}{2} = \frac{15}{2} \delta c^2$
 $q_1 = \frac{V \bar{S}_1}{I_y} = \frac{\frac{2P}{3} \times \frac{15}{2} \delta c^2}{56 \delta c^3} = \frac{5}{56} \frac{P}{c}$ força longitudinal por unidade de comprimento que deve ser resistida por P1
 resultante em A1 área efetiva
 $q_1 \times A_1 = \bar{\sigma}_{pm} \times A_{ef} = \bar{\sigma}_{pm} \times (2A_1)$
 $A_1 = \frac{q_1 \times 10}{2 \times 7,5} = \frac{(\frac{5}{56} \times \frac{150}{10}) \times 10}{2 \times 7,5} = \frac{50}{56} = 0,89 \text{ cm}^2$ ($R = 0,53 \text{ cm}$)

Parafuso P2



$\bar{S}_2 = \bar{S}_1 + 2(c\delta \times 2c + 2c\delta \cdot c) = (\frac{15}{2} + \frac{16}{2}) \delta c^2 = \frac{31}{2} \delta c^2$
 $q_2 = \frac{V \bar{S}_2}{I_y} = \frac{\frac{2P}{3} \times \frac{31}{2} \delta c^2}{56 \delta c^3} = \frac{31}{168} \frac{P}{c}$
 $q_2 \times 2 = \bar{\sigma}_{pm} (4A_2) \Rightarrow A_2 = 0,92 \text{ cm}^2$ ($R = 0,54 \text{ cm}$)

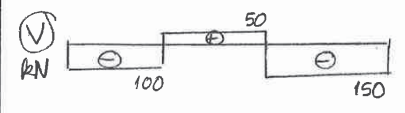
Ponto M

$\bar{S}_y^M = (c\delta) \times 2c = 2\delta c^2$
 $\sigma^M = \frac{V \bar{S}_y^M}{b I_y} = \frac{\frac{2P}{3} \times 2\delta c^2}{b \times 56 \delta c^3} = \frac{P}{42\delta c} = 0,71 \frac{\text{kN}}{\text{cm}^2}$

96P3Q2 (Ex. 2)

Suporte man. admissível:

$\uparrow \{ R_B + R_D = 300$
 $\sum B \{ -100 \times 2 + 500 + 200 \times 3 - R_D \times 6 = 0$
 $6R_D = 900 \Rightarrow R_D = 150$
 $R_B = 150$
 verif.
 $\sum D \{ -100 \times 8 + 150 \times 6 - 200 \times 3 + 500 = 0$



Parafusos (trecho CD)

$P_1) \bar{S}_1 = (24 \times 1) \times 13,5 = 468 \text{ cm}^3$
 $q_1 = \frac{V \bar{S}_1}{I_y} = \frac{150 \times 468}{47936} = 1,464 \frac{\text{kN}}{\text{cm}}$
 $q_1 a_1 = 2\bar{\sigma} \Rightarrow a_1 = \frac{2 \times 9,425}{1,464} = 12,9 \text{ cm}$
 $nf = \frac{300}{12,9} = 23,3$ Adotando 2x 24 paraf. $a_1^* = 12,5 \text{ cm}$

$P_2) \bar{S}_2 = (14 \times 1) \times 18,5 + 2((1 \times 6) \times 15) = 439 \text{ cm}^3$
 $q_2 = \frac{V \bar{S}_2}{I_y} = \frac{150 \times 439}{47936} = 1,374 \frac{\text{kN}}{\text{cm}}$
 $q_2 a_2 = 2\bar{\sigma} \Rightarrow a_2 = 13,7 \text{ cm}$
 $nf = \frac{300}{13,7} = 21,9$ Adotando 2x 22 paraf. $a_2^* = 13,6 \text{ cm}$

$P_3) \bar{S}_3 = \bar{S}_1 + \bar{S}_2 = 468 + 439 = 907 \text{ cm}^3$
 $q_3 = \frac{150 \times 907}{47936} = 2,838 \frac{\text{kN}}{\text{cm}}$
 $q_3 a_3 = 2\bar{\sigma} \Rightarrow a_3 = 6,64 \text{ cm}$
 $nf = \frac{300}{6,64} = 45,2$ Adotando 2x 46 paraf. $a_3^* = 6,5 \text{ cm}$

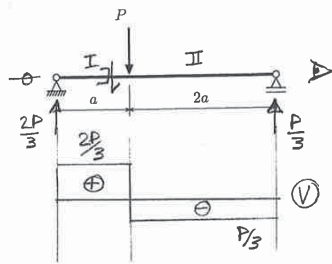
$P_4) \bar{S}_4 = \bar{S}_1$
 $a_4^* = a_1^* = 12,5 \text{ cm}$

Solução prática considerando a fabricação do perfil.

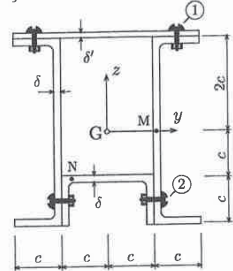
- P1: 48 parafusos $a = 12,5 \text{ cm}$
- P2: 48 " $a = 12,5 \text{ cm}$
- P3: 96 " $a = 6,25 \text{ cm}$
- P4: 48 " $a = 12,5 \text{ cm}$

98P5Q2

Ex. 3



Seção Transversal:



$$b' = \frac{5}{8} b \text{ (chapa sup.)}$$

a) Espaçamentos longitudinais

$$P_1: \bar{S}_1 = 4c \left(\frac{5}{8}b\right) 2c = 5b^2c^2 \Rightarrow q_1 = \frac{2P}{3} \frac{5b^2c^2}{130b^2c^3} = \frac{P}{13c}$$

$$q_1 a_1 = \bar{\sigma}_p (2Ap) \Rightarrow a_1 = \frac{2\bar{\sigma}_p A_p \cdot 13c}{P} = \frac{2 \times 75000 \times 15}{45000} \times 13 \times 15 = 74750 \text{ cm} \quad (a_1 = 750 \text{ cm})$$

$$a_1^* = 7,5 \text{ cm} \begin{cases} 80 \text{ parafusos em I} \\ 80 \text{ parafusos em II} \end{cases} \quad (\bar{a}_1 = 150 \text{ cm})$$

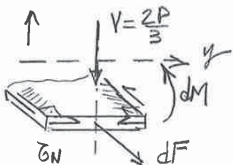
$$P_2: \bar{S}_2 = (2cb)c + 2(c^2 \times 15c) = 5b^2c^2$$

Como $\bar{S}_2 = \bar{S}_1$, $a_2^* = 7,5 \text{ cm}$

$$\begin{cases} 80 \text{ parafusos em I } (a_2 = 7,5 \text{ cm}) \\ 80 \text{ parafusos em II} \end{cases} \quad (\bar{a}_2 = 150 \text{ cm})$$

b) Tensões tangenciais nos pontos M e N

Ponto N (forçamb 2 centros longitudinais simétricos)



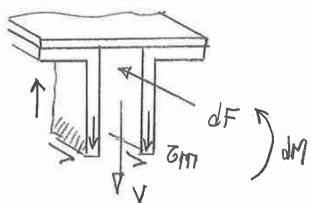
$$\bar{S}_N = (2cb)c = 2b^2c^2$$

$$\bar{\sigma}_N = \frac{V \bar{S}_N}{b I_y} = \frac{\frac{2P}{3} \times 2b^2c^2}{(2b) \times \frac{130b^2c^3}{3}} = \frac{P}{65bc} = \frac{45000}{65 \times 12 \times 15}$$

2 seções com tamar $\bar{\sigma}_m = \bar{\sigma}$

$$\bar{\sigma}_N = 38,5 \frac{\text{kgf}}{\text{cm}^2} \quad (\rightarrow)$$

Ponto M (forçamb 2 centros longitudinais simétricos)



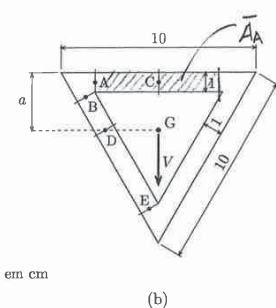
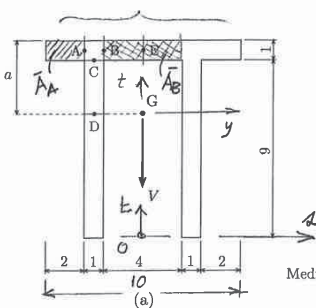
$$\bar{S}_M = \bar{S}_1 + 2(c^2 \times 2c + 2c^2c) = 13b^2c^2$$

$$\bar{\sigma}_M = \frac{\frac{2P}{3} \times 13b^2c^2}{(2b) \times \frac{130b^2c^3}{3}} = \frac{P}{10bc} = \frac{45000}{10 \times 12 \times 15} = 2500 \frac{\text{kgf}}{\text{cm}^2} \quad (\downarrow)$$

2014 nov 13

Ex. 4

PEF2201 L7-1

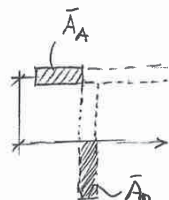


Medidas em cm

Seção (a)

$$t_G = \frac{10 \times 10 \times 5 - 8 \times 9 \times 4,5}{100 - 72} = 6,2857 \text{ cm} \Rightarrow a = 3,7143 \text{ cm}$$

$$I_{x_2} = \frac{10 \times 10^3 - 8 \times 9^3}{3} = 1389,33 \text{ cm}^4 \Rightarrow I_y = I_{x_2} - 28 \times t_G^2 = 283,05 \text{ cm}^4$$



$$\bar{S}_A = (2 \times 1) \times 3,214 = 6,428 \text{ cm}^3$$

$$\bar{S}_B = (4 \times 1) \times 3,214 = 12,856 \text{ cm}^3$$

$$\bar{S}_C = (10 \times 1) \times 3,214 = 32,14 \text{ cm}^3$$

$$\bar{S}_D = -\frac{1 \times 6,2857^2}{2} = -19,75 \text{ cm}^3$$

$$\bar{S}_E = 0$$

$$\bar{\sigma} = \frac{V \bar{S}_y}{b I_y} = 3,533 \times 10^{-3} \frac{V \bar{S}_y}{b}$$

$$\Rightarrow \bar{\sigma}_A = \sqrt{\bar{S}_A} / (1 \times I_y) = 2,27 \times 10^{-2} V$$

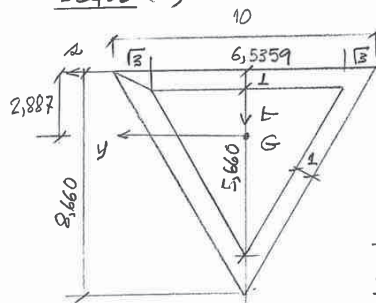
$$\bar{\sigma}_B = \sqrt{\bar{S}_B} / (2 \times I_y) = 2,27 \times 10^{-2} V$$

$$\bar{\sigma}_C = \sqrt{\bar{S}_C} / (2 \times I_y) = 5,68 \times 10^{-2} V$$

$$\bar{\sigma}_D = \sqrt{\bar{S}_D} / (1 \times I_y) = 6,98 \times 10^{-2} V$$

$$\Rightarrow \bar{\sigma}_E = 0$$

Seção (b)



Como os triângulos são concêntricos:

$$t_G = \frac{8,660}{3} = 2,887 \text{ cm} = a$$

$$I_{x_2} = \frac{10 \times 8,66^3}{12} - \frac{(6,5359 \times 5,660^3 + 18,50 \times 2,887^2)}{36} = 354,10 \text{ cm}^4$$

$$I_y = I_{x_2} - 24,80 \times (2,887)^2 = 147,4 \text{ cm}^4$$

$$\bar{S}_A = 6,5359 \times 2,327 = 15,60 \text{ cm}^3$$

$$\bar{S}_B \approx (10 \times 1) \times 2,327 = 23,27 \text{ cm}^3$$

$$\bar{S}_C = 0$$

$$\bar{S}_D \approx \bar{S}_B + 2(1,887 \times 2) \times \left(\frac{1,887}{2}\right) = 27,43 \text{ cm}^3$$

$$\bar{S}_E \approx 2\left(\frac{13 \times 1}{2}\right) \left(\frac{2}{3} \times 2,866 - \frac{2}{3}\right) = 8,84 \text{ cm}^3$$

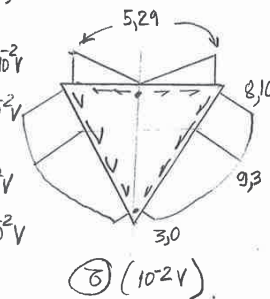
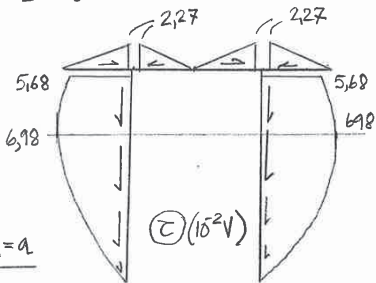
$$\Rightarrow \bar{\sigma}_A = \sqrt{\bar{S}_A} / (2 I_y) = 5,29 \times 10^{-2} V$$

$$\bar{\sigma}_B = \sqrt{\bar{S}_B} / (2 I_y) = 8,10 \times 10^{-2} V$$

$$\bar{\sigma}_C = 0$$

$$\bar{\sigma}_D = \sqrt{\bar{S}_D} / (2 I_y) = 9,3 \times 10^{-2} V$$

$$\bar{\sigma}_E = \sqrt{\bar{S}_E} / (2 I_y) = 3,0 \times 10^{-2} V$$



* compenna calcular o mom. de inércia em relação a y'

$$I_{y'} = \frac{10 \times 8,66^3}{36} - \frac{6,5359 \times 5,66^3}{36} = 147,5 \text{ cm}^4 \quad \checkmark$$

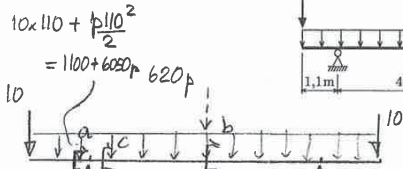
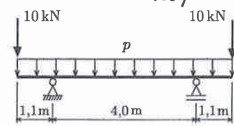
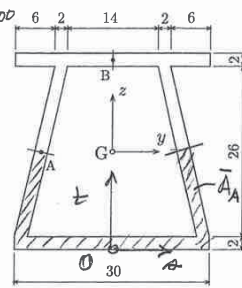
PEF 2201 L7-2

Ex. 5

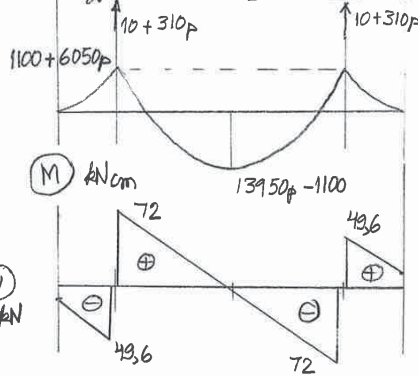
$$M = -10 \times 310 - p \frac{310^2}{2} + (10 + 310p) 200$$

$$= -1100 + 13950p$$

Seção Transversal: (cm)



$p \left[\frac{kN}{cm} \right]$



$t_G = 15 \text{ cm}$ (abrirá os parafusos)

$$I_y = \frac{30 \times 30^3}{12} - \frac{26 \times 26^3}{12} = 29418,7 \text{ cm}^4$$

$$W' = W'' = \frac{I_y}{15} = 1961 \text{ cm}^3$$

Como $\bar{\sigma}_T = 5 \frac{kN}{cm^2}$ e $\bar{\sigma}_C = 2 \frac{kN}{cm^2}$

as fibras mais tensionadas nos eixos a-a e b-b são críticas.

$$\sigma_{at} = \frac{1100 + 6050p}{1961} \leq 2 \Rightarrow p \leq 0,466 \frac{kN}{cm}$$

$$\sigma_{bt} = \frac{13950 - 1100}{1961} \leq 2 \Rightarrow p \leq 0,360 \frac{kN}{cm}$$

$p = 0,36 \frac{kN}{cm}$

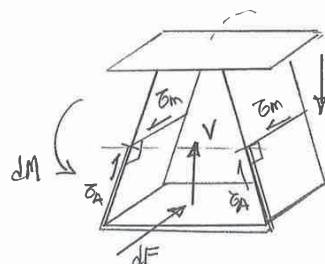
Tensões tangenciais:

Seção c-c: ($V = 72 \text{ kN}$)

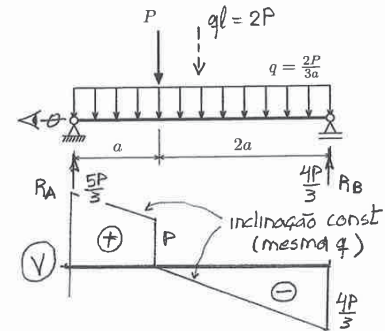
$$\bar{S}_A = (30 \times 15) \frac{15}{2} - (26 \times 13) \frac{13}{2} = 1178 \text{ cm}^3$$

$$\bar{\sigma}_A = \frac{72 \times 1178}{(2 \times 195) 29418,7} = 0,739 \frac{kN}{cm^2} (\uparrow)$$

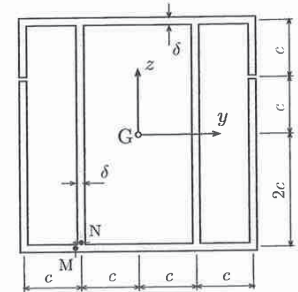
$\sigma_B = 0$ (simetria)



Para a seção mais solicitada à força cortante da viga da figura, determine as tensões tangenciais nos pontos M e N segundo o ponto de vista indicado na figura.



Seção Transversal:



a) Reações

$$\uparrow \{ R_A + R_B = 3P$$

$$\curvearrowleft \{ R_A \cdot 3a - P \cdot 2a - 2P \cdot \frac{3a}{2} = 0$$

$$R_A = \frac{5P}{3} \quad R_B = \frac{4P}{3}$$

$$\text{verif } \curvearrowright \{ P_A + 2P \cdot \frac{3a}{2} - \frac{4P}{3} \cdot 3a = 0$$

b) Momento de inércia ($\delta \ll c$)

$$I_y = 2 \left(\frac{4c}{3} \delta \right) + 4 \left(\frac{\delta}{4} \right) (4c)$$

$$= 2(4\delta c (2c)^2) + 4 \left(\frac{\delta (4c)^3}{12} \right)$$

$$= (32 + \frac{64}{3}) \delta c^3 = \frac{160}{3} \delta c^3$$

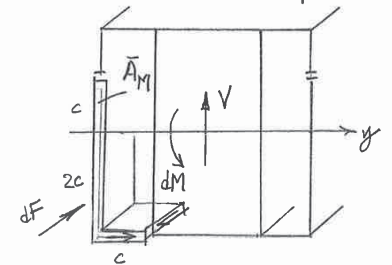
c) Tensão tangencial em M

$$\bar{S}_M = (3c\delta) \left(-\frac{c}{2}\right) + c\delta(-2c) = -\frac{7}{2} \delta c^2$$

$$\bar{c} = \frac{V \bar{S}}{b I_y} = \frac{\frac{5P}{3} \times \frac{7}{2} \delta c^2}{b \frac{160}{3} \delta c^3} = \frac{P}{32\delta c^2} \frac{7}{b}$$

$$\bar{\sigma}^M = \frac{P}{32\delta c^2} \frac{7}{b} \frac{7}{2} \delta c^2 = \frac{7}{64} \frac{P}{bc} (\rightarrow)$$

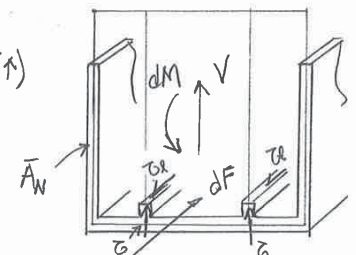
seção vista da esquerda



d) Tensão tangencial em N

$$\bar{S} = \sqrt{2} (3c\delta) \left(-\frac{c}{2}\right) + (4c\delta) (-2c) = -11 \delta c^2$$

$$\bar{\sigma}^N = \frac{P}{32\delta c^2} \frac{7}{b} = \frac{P}{32\delta c^2} \frac{11 \delta c^2}{(2\delta)} = \frac{11}{64} \frac{P}{bc} (\uparrow)$$



yy - Diogo Ex. 7

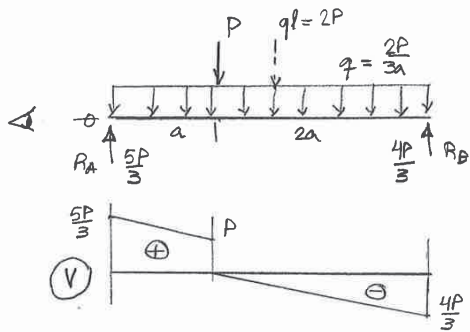
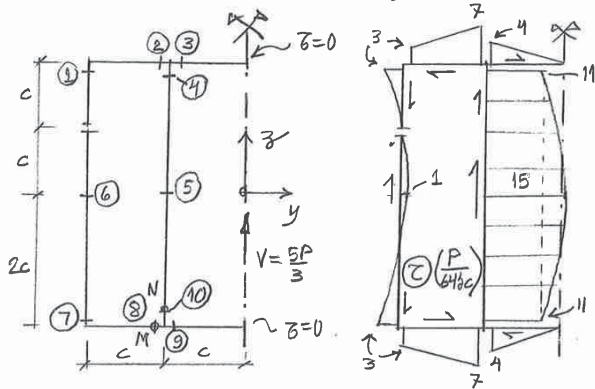


Diagrama de G considerando a simetria:

Cortes longitudinais:

Diagrama de G



$$\begin{aligned} \bar{s}_1 &= bc \cdot \frac{3}{2}c = \frac{3}{2}bc^2 \\ \bar{s}_2 &= \bar{s}_1 + bc \cdot 2c = \frac{7}{2}bc^2 \\ \bar{s}_3 &= (bc) \cdot 2c = 2bc^2 \\ \bar{s}_4 &= \bar{s}_2 + \bar{s}_3 = \frac{11}{2}bc^2 \\ \bar{s}_5 &= \bar{s}_4 + 2bc \cdot c = \frac{15}{2}bc^2 \\ \bar{s}_6 &= bc \cdot \frac{c}{2} = \frac{1}{2}bc^2 \\ \bar{s}_7 &= (3bc) \cdot (-\frac{c}{2}) = -\frac{3}{2}bc^2 \\ \bar{s}_8 &= \bar{s}_2 + bc \cdot (-2c) = -\frac{3}{2}bc^2 \\ \bar{s}_9 &= -\bar{s}_3 = -2bc^2 \\ \bar{s}_{10} &= -\bar{s}_4 = -\frac{11}{2}bc^2 \end{aligned}$$

$$\begin{aligned} \tau_1 &= \frac{V\bar{s}}{I_y} = \frac{\frac{5P}{3} \times \frac{7}{2}bc^2}{\frac{160}{3}bc^3} = \frac{7P}{32bc} \\ \tau_2 &= \frac{7}{64} \frac{P}{bc} \\ \tau_3 &= \frac{4}{64} \frac{P}{bc} \\ \tau_4 &= \frac{1}{64} \frac{P}{bc} \\ \tau_5 &= \frac{15}{64} \frac{P}{bc} \\ \tau_6 &= \frac{1}{64} \frac{P}{bc} \\ \tau_7 &= \frac{3}{64} \frac{P}{bc} \\ \tau_8 &= \frac{7}{64} \frac{P}{bc} \\ \tau_9 &= \frac{4}{64} \frac{P}{bc} \\ \tau_{10} &= \frac{1}{64} \frac{P}{bc} \end{aligned}$$

$A = (2 \times 4 + 4 \times 4)bc = 24bc$
 $\tau_5 = \frac{45}{8} \frac{P}{A}$
 Tensões bem mais elevadas que $\tau_m = P/A$.

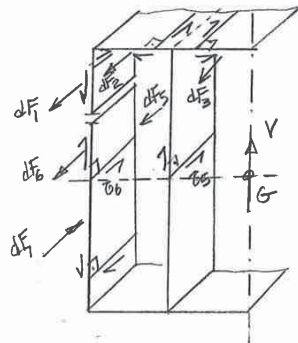
a) Reações

$$\begin{aligned} \uparrow \{ R_A + R_B &= 3P \\ \curvearrowright B \{ R_A \cdot 3a - P \cdot 2a - 2P \cdot \frac{3a}{2} &= 0 \\ R_A &= \frac{5P}{3} \quad R_B = \frac{4P}{3} \end{aligned}$$

b) Momento de inércia (seção delgada)

$$I_y = 2[4bc(2c)^2] + 4[\frac{b(4c)^3}{12}] = (32 + \frac{64}{3})bc^3 = \frac{160}{3}bc^3$$

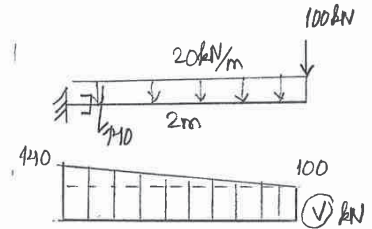
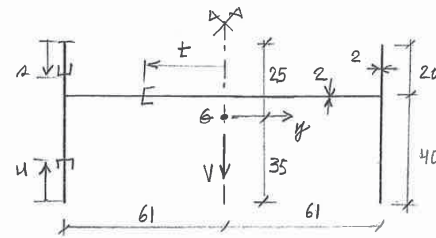
Determinação dos sentidos



04P3Q2 Ex. 8

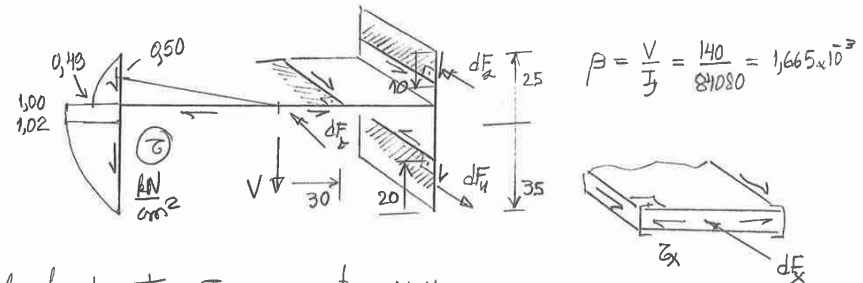
a) Distribuição das tensões na seção da engate

Considerando a ST delgada



$$q = \frac{V\bar{s}_y}{I_y} = \beta \bar{s}_y \quad \tau = \frac{q}{b}$$

$$\begin{aligned} \bar{s}(a) = 2a \times (25 - \frac{a}{2}) &\Rightarrow q(a) \begin{cases} a=0 & q=0 \\ a=19 & q=589\beta \end{cases} \quad \tau = \frac{q}{b} \begin{cases} \tau=0 \\ \tau = \frac{589\beta}{2} = 294,5\beta \end{cases} \\ \bar{s}(u) = 2u \times (35 - \frac{u}{2}) &\Rightarrow q(u) \begin{cases} u=0 & q=0 \\ u=35 & q=1225\beta \\ u=39 & q=1209\beta \end{cases} \quad \tau = \frac{q}{b} \begin{cases} \tau=0 \\ \tau = 612,5\beta \\ \tau = 604,5\beta \end{cases} \\ \bar{s}(t) = 2(2t) \times 5 = 20t &\Rightarrow q(t) \begin{cases} t=0 & q=0 \\ t=60 & q=1200\beta \end{cases} \quad \tau = \frac{q}{b} \begin{cases} \tau=0 \\ \tau = \frac{1200\beta}{4} = 300\beta \end{cases} \end{aligned}$$



b) Cálculo das tensões nos pontos X, Y e Z

$$\begin{aligned} \bar{s}_x &= 2(30 \times 2) \times 5 = 600 \text{ cm}^3 \Rightarrow \tau_x = \frac{140 \times 600}{(2 \times 2) \times 84080} = 0,25 \frac{\text{kN}}{\text{cm}^2} (\leftarrow) \\ \bar{s}_y &= (2 \times 10) \times 20 = 400 \text{ cm}^3 \Rightarrow \tau_y = \frac{\beta \times 400}{2} = 0,33 \frac{\text{kN}}{\text{cm}^2} (\downarrow) \\ \bar{s}_z &= (2 \times 20) \times 25 = 1000 \text{ cm}^3 \Rightarrow \tau_z = \frac{\beta \times 1000}{2} = 0,83 \frac{\text{kN}}{\text{cm}^2} (\downarrow) \end{aligned}$$

