

Chapter 2

BASIC SOLUTIONS FOR CONCENTRATED LOADING

2.1 Point Loading

2.1.1 KELVIN PROBLEM

Point load acting within an infinite elastic mass (Fig.2.1).

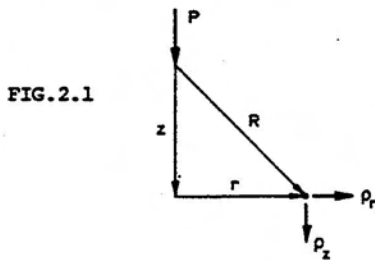


FIG. 2.1

$$\sigma_z = \frac{P}{8\pi(1-\nu)} \left[\frac{3z^3}{R^5} + \frac{(1-2\nu)z}{R^3} \right] \quad \dots (2.1a)$$

$$\sigma_r = \frac{P}{8\pi(1-\nu)} \frac{z}{R^3} \left[\frac{3r^2}{R^2} - (1-2\nu) \right] \quad \dots (2.1b)$$

$$\sigma_\theta = -\frac{P(1-2\nu)}{8\pi(1-\nu)} \frac{z}{R^3} \quad \dots (2.1c)$$

$$\theta = \frac{P}{8\pi(1-\nu)} \cdot \frac{2(1+\nu)z}{R^3} \quad \dots (2.1d)$$

$$\tau_{rz} = \frac{P}{8\pi(1-\nu)} \frac{r}{R^3} \left[\frac{3z^2}{R^2} + (1-2\nu) \right] \quad \dots (2.1e)$$

$$\rho_z = \frac{P(1+\nu)}{8\pi(1-\nu)ER} \left[3 - 4\nu + \frac{z^2}{R^2} \right] \quad \dots (2.1f)$$

$$\rho_r = -\frac{P(1+\nu)}{8\pi(1-\nu)E} \cdot \frac{rz}{R^3} \quad \dots (2.1g)$$

2.1.2 BOUSSINESQ PROBLEM

Point load acting on the surface of a semi-infinite mass (Fig.2.2).

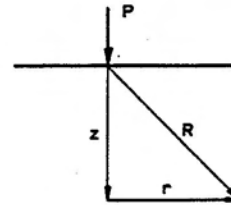


FIG. 2.2

$$\sigma_z = \frac{3Pz^3}{2\pi R^5} \quad \dots (2.2a)$$

$$\sigma_r = -\frac{P}{2\pi R^2} \left[\frac{-3r^2 z}{R^3} + \frac{(1-2\nu)R}{R+z} \right] \quad \dots (2.2b)$$

$$\sigma_\theta = -\frac{(1-2\nu)P}{2\pi R^2} \left[\frac{z}{R} - \frac{R}{R+z} \right] \quad \dots (2.2c)$$

$$\theta = \frac{(1+\nu)Pz}{\pi R^3} \quad \dots (2.2d)$$

$$\tau_{rz} = \frac{3Prz^2}{2\pi R^5} \quad \dots (2.2e)$$

$$\rho_z = \frac{P(1+\nu)}{2\pi ER} \left[2(1-\nu) + \frac{z^2}{R^2} \right] \quad \dots (2.2f)$$

$$\rho_r = \frac{P(1+\nu)}{2\pi ER} \left[\frac{rz}{R^2} - \frac{(1-2\nu)r}{R+z} \right] \quad \dots (2.2g)$$

2.1.3 CERUTTI'S PROBLEM

Horizontal point load acting along the surface of a semi-infinite mass (Fig.2.3).

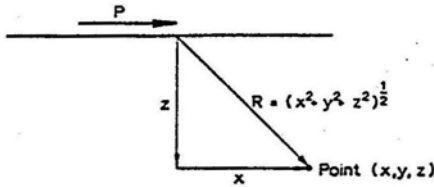


FIG. 2.3

$$\sigma_z = \frac{3Pxz^2}{2\pi R^5} \quad \dots (2.3a)$$

$$\sigma_x = \frac{-Px}{2\pi R^3} \left[\frac{-3x^2}{R^2} + \frac{1-2\nu}{(R+z)^2} \left(R^2 - y^2 - \frac{2Ry^2}{R+z} \right) \right] \quad \dots (2.3b)$$

$$\sigma_y = \frac{-Px}{2\pi R^3} \left[\frac{-3y^2}{R^2} + \frac{1-2\nu}{(R+z)^2} \left(3R^2 - x^2 - \frac{2Rx^2}{R+z} \right) \right] \quad \dots (2.3c)$$

$$\theta = \frac{(1+\nu)Px}{\pi R^3} \quad \dots (2.3d)$$

$$\tau_{xy} = \frac{-Py}{2\pi R^3} \left[-\frac{3x^2}{R^2} + \frac{(1-2\nu)}{(R+z)^2} \left(-R^2 + x^2 + \frac{2Rx^2}{R+z} \right) \right] \quad \dots (2.3e)$$

$$\tau_{yz} = \frac{3Pxyz}{2\pi R^5} \quad \dots (2.3f)$$

$$\tau_{zx} = \frac{3Px^2z}{2\pi R^5} \quad \dots (2.3g)$$

$$\rho_z = \frac{P(1+\nu)}{2\pi ER} \left[\frac{zx}{R^2} + \frac{(1-2\nu)x}{R+z} \right] \quad \dots (2.3h)$$

$$\rho_x = \frac{P(1+\nu)}{2\pi ER} \left[1 + \frac{x^2}{R^2} + (1-2\nu) \left(\frac{R}{R+z} - \frac{x^2}{(R+z)^2} \right) \right] \quad \dots (2.3i)$$

$$\rho_y = \frac{P(1+\nu)}{2\pi ER} \left[\frac{xy}{R^2} - \frac{(1-2\nu)xy}{(R+z)^2} \right] \quad \dots (2.3j)$$

2.1.4 MINDLIN'S PROBLEM NO.1

Vertical point load P acting beneath the surface of a semi-infinite mass. (Mindlin, 1936). (Fig.2.4).

$$\sigma_x = \frac{-P}{8\pi(1-\nu)} \left[\frac{(1-2\nu)(z-c)}{R_1^3} - \frac{3x^2(z-c)}{R_1^5} + \frac{(1-2\nu)[3(z-c)-4\nu(z+c)]}{R_2^3} \right]$$

$$\begin{aligned} & - \frac{3(3-4\nu)x^2(z-c)-6c(z+c)[(1-2\nu)z-2\nu c]}{R_2^5} \\ & - \frac{30cx^2z(z+c)}{R_2^7} - \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)} \times \\ & \times \left(1 - \frac{x^2}{R_2(R_2+z+c)} - \frac{z^2}{R_2^2} \right) \quad \dots (2.4a) \end{aligned}$$

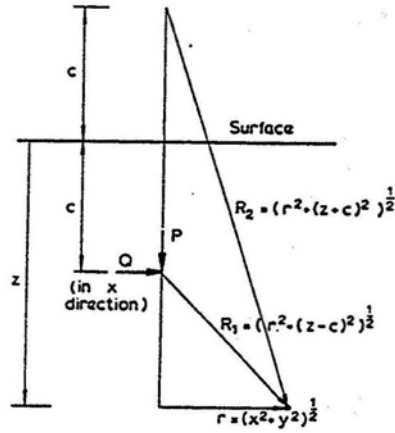


FIG. 2.4

$$\begin{aligned} \sigma_y = & \frac{-P}{8\pi(1-\nu)} \left[\frac{(1-2\nu)(z-c)}{R_1^3} - \frac{3y^2(z-c)}{R_1^5} \right. \\ & + \frac{(1-2\nu)[3(z-c)-4\nu(z+c)]}{R_2^3} \\ & - \frac{3(3-4\nu)y^2(z-c)-6c(z+c)[(1-2\nu)z-2\nu c]}{R_2^5} \\ & - \frac{30cy^2z(z+c)}{R_2^7} - \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)} \times \\ & \left. \times \left(1 - \frac{y^2}{R_2(R_2+z+c)} - \frac{z^2}{R_2^2} \right) \right] \quad \dots (2.4b) \end{aligned}$$

$$\begin{aligned} \sigma_z = & \frac{-P}{8\pi(1-\nu)} \left[-\frac{(1-2\nu)(z-c)}{R_1^3} + \frac{((1-2\nu)(z-c))}{R_2^3} \right. \\ & - \frac{3(z-c)^3}{R_1^5} - \frac{3(3-4\nu)z(z+c)^2-3c(z+c)(5z-c)}{R_2^5} \\ & \left. - \frac{30cz(z+c)^3}{R_2^7} \right] \quad \dots (2.4c) \end{aligned}$$

$$\tau_{yz} = \frac{-Py}{8\pi(1-\nu)} \left[-\frac{(1-2\nu)}{R_1^3} + \frac{1-2\nu}{R_2^3} - \frac{3(z-c)^2}{R_1^5} - \frac{3(3-4\nu)z(z+c)-3c(3z+c)}{R_2^5} - \frac{30cz(z+c)^2}{R_2^7} \right] \dots (2.4d)$$

$$\tau_{zx} = \frac{-Px}{8\pi(1-\nu)} \left[-\frac{1-2\nu}{R_1^3} + \frac{1-2\nu}{R_2^3} - \frac{3(z-c)^2}{R_1^5} - \frac{3(3-4\nu)z(z+c)-3c(3z+c)}{R_2^5} - \frac{30cz(z+c)^2}{R_2^7} \right] \dots (2.4e)$$

$$\tau_{xy} = \frac{-Pxy}{8\pi(1-\nu)} \left[-\frac{3(z-c)}{R_1^5} - \frac{3(3-4\nu)(z-c)}{R_2^5} + \frac{4(1-\nu)(1-2\nu)}{R_2^2(R_2+z+c)} \left(\frac{1}{R_2+z+c} + \frac{1}{R_2} \right) - \frac{30cz(z+c)}{R_2^7} \right] \dots (2.4f)$$

$$\sigma_r = \frac{-P}{8\pi(1-\nu)} \left[\frac{(1-2\nu)(z-c)}{R_1^3} - \frac{(1-2\nu)(z+c)}{R_2^3} + \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)} - \frac{3r^2(z-c)}{R_1^5} + \frac{6c(1-2\nu)(z+c)^2-6c^2(z+c)-3(3-4\nu)r^2(z-c)}{R_2^5} - \frac{30cr^2z(z+c)}{R_2^7} \right] \dots (2.4g)$$

$$\sigma_\theta = \frac{-P(1-2\nu)}{8\pi(1-\nu)} \left[\frac{z-c}{R_1^3} + \frac{(3-4\nu)(z+c)-6c}{R_2^3} - \frac{4(1-\nu)}{R_2(R_2+z+c)} + \frac{6c(z+c)^2}{R_2^5} - \frac{6c^2(z+c)}{(1-2\nu)R_2^5} \right] \dots (2.4h)$$

$$\tau_{rz} = \frac{-Pr}{8\pi(1-\nu)} \left[-\frac{1-2\nu}{R_1^3} + \frac{1-2\nu}{R_2^3} - \frac{3(z-c)^2}{R_1^5} - \frac{3(3-4\nu)z(z+c)-3c(3z+c)}{R_2^5} - \frac{30cz(z+c)^2}{R_2^7} \right] \dots (2.4i)$$

$$\rho_r = \frac{Pr}{16\pi G(1-\nu)} \left[\frac{z-c}{R_1^3} + \frac{(3-4\nu)(z-c)}{R_2^3} - \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)} + \frac{6cz(z+c)}{R_2^5} \right] \dots (2.4j)$$

$$\rho_z = \frac{P}{16\pi G(1-\nu)} \left[\frac{3-4\nu}{R_1} + \frac{8(1-\nu)^2-(3-4\nu)}{R_2} + \frac{(z-c)^2}{R_1^3} + \frac{(3-4\nu)(z+c)^2-2cz}{R_2^3} + \frac{6cz(z+c)^2}{R_2^5} \right] \dots (2.4k)$$

Influence factors for σ_z , and σ_r and σ_θ on the axis have been tabulated by Geddes (1966).

2.1.5 MINDLIN'S PROBLEM NO.2.

Horizontal point load Q acting beneath the surface of a semi-infinite mass. (Mindlin, 1936). (Fig.2.4).

$$\sigma_x = \frac{-Qx}{8\pi(1-\nu)} \left[-\frac{(1-2\nu)}{R_1^3} + \frac{(1-2\nu)(5-4\nu)}{R_2^3} - \frac{3x^2}{R_1^5} - \frac{3(3-4\nu)x^2}{R_2^5} - \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)^2} \times \left(3 - \frac{x^2(3R_2+z+c)}{R_2^2(R_2+z+c)} \right) + \frac{6c}{R_2^2} \left(3c - (3-2\nu)(z+c) + \frac{5x^2z}{R_2^2} \right) \right] \dots (2.5a)$$

$$\sigma_y = \frac{-Qx}{8\pi(1-\nu)} \left[\frac{(1-2\nu)}{R_1^3} + \frac{(1-2\nu)(3-4\nu)}{R_2^3} - \frac{3y^2}{R_1^5} - \frac{3(3-4\nu)y^2}{R_2^5} - \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)^2} \times \left(1 - \frac{y^2(3R_2+z+c)}{R_2^2(R_2+z+c)} \right) \right] \dots (2.5b)$$

(Continued)

$$+ \frac{6c}{R_2^5} \left(c - (1-2\nu)(z+c) + \frac{5y^2z}{R_2^2} \right) \quad \dots (2.5b)$$

$$\sigma_z = \frac{Qx}{8\pi(1-\nu)} \left[\frac{(1-2\nu)}{R_1^3} - \frac{(1-2\nu)}{R_2^3} - \frac{3(z-c)^2}{R_1^5} \right. \\ \left. - \frac{3(3-4\nu)(z+c)^2}{R_2^5} \right. \\ \left. + \frac{6c}{R_2^5} \left(c + (1-2\nu)(z+c) + \frac{5z(z+c)^2}{R_2^2} \right) \right] \quad \dots (2.5c)$$

$$\tau_{yz} = \frac{-Qxy}{8\pi(1-\nu)} \left[-\frac{3(z-c)}{R_1^5} - \frac{3(3-4\nu)(z+c)}{R_2^5} \right. \\ \left. + \frac{6c}{R_2^5} \left(1 - 2\nu + \frac{5z(z+c)}{R_2^2} \right) \right] \quad \dots (2.5d)$$

$$\tau_{zx} = \frac{-Q}{8\pi(1-\nu)} \left[-\frac{(1-2\nu)(z-c)}{R_1^3} + \frac{(1-2\nu)(z-c)}{R_2^3} \right. \\ \left. - \frac{3x^2(z-c)}{R_1^5} - \frac{3(3-4\nu)x^2(z+c)}{R_2^5} \right. \\ \left. - \frac{6c}{R_2^5} \left(z(z+c) - (1-2\nu)x^2 - \frac{5x^2z(z+c)}{R_2^2} \right) \right] \quad \dots (2.5e)$$

$$\tau_{xy} = \frac{-Qy}{8\pi(1-\nu)} \left[-\frac{(1-2\nu)}{R_1^3} + \frac{(1-2\nu)}{R_2^3} - \frac{3x^2}{R_1^5} \right. \\ \left. - \frac{3(3-4\nu)x^2}{R_2^5} - \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)^2} \times \right. \\ \left. \times \left(1 - \frac{x^2(3R_2+z+c)}{R_2^2(R_2+z+c)} \right) - \frac{6cz}{R_2^5} \left(1 - \frac{5x^2}{R_2^2} \right) \right] \quad \dots (2.5f)$$

$$\rho_x = \frac{Q}{16\pi G(1-\nu)} \left[\frac{(3-4\nu)}{R_1} + \frac{1}{R_2} + \frac{x^2}{R_1^3} + \frac{(3-4\nu)x^2}{R_2^3} \right. \\ \left. + \frac{2cz}{R_2^3} \left(1 - \frac{3x^2}{R_2^2} \right) + \frac{4(1-\nu)(1-2\nu)}{R_2+z+c} \times \right. \\ \left. \times \left(1 - \frac{x^2}{R_2(R_2+z+c)} \right) \right] \quad \dots (2.5g)$$

$$\rho_y = \frac{Qxy}{16\pi G(1-\nu)} \left[\frac{1}{R_1^3} + \frac{(3-4\nu)}{R_2^3} - \frac{6cz}{R_2^5} \right. \\ \left. - \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)^2} \right] \quad \dots (2.5h)$$

$$\rho_z = \frac{Qx}{16\pi G(1-\nu)} \left[\frac{z-c}{R_1^3} + \frac{(3-4\nu)(z-c)}{R_2^3} \right. \\ \left. - \frac{6cz(z+c)}{R_2^5} + \frac{4(1-\nu)(1-2\nu)}{R_2(R_2+z+c)} \right] \quad \dots (2.5i)$$

2.1.6 POINT LOAD ON FINITE LAYER

Vertical point load acting at the surface of a layer underlain by a rough rigid base (Fig.2.5). This problem has been studied in detail by Burmister (1943,1945).

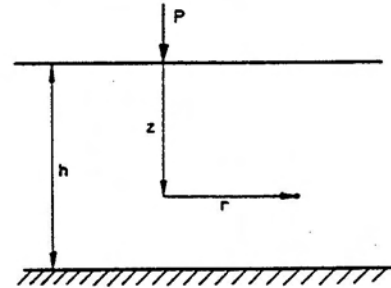


FIG. 2.5

Numerical values for the stresses and displacements in this problem have been tabulated by Poulos (1967b), and are given in TABLES 2.1 to 2.7.

2.1.7 OTHER SOLUTIONS

(a) *Point Loads Inside an Infinite Two Layer System* - Solutions for stresses and displacements have been given by Plevako (1969) for both vertical and horizontal point loading.

(b) *Point Load Within an Elastic Layer* - A formal solution for stresses and displacements has been given by Shekter and Prikhodchenko (1964), but no numerical values are evaluated.

TABLE 2.5
INFLUENCE FACTORS $I_{\tau_{rz}}$ FOR SHEAR STRESS τ_{rz}

POINT LOAD

$$\tau_{rz} = I_{\tau_{rz}} \frac{P}{2\pi h^2}$$

z/h		I _{τ_{rz}} for Poisson's Ratio ν = 0.0										I _{τ_{rz}} for Poisson's Ratio ν = 0.2										I _{τ_{rz}} for Poisson's Ratio ν = 0.4										I _{τ_{rz}} for Poisson's Ratio ν = 0.5									
		1.0	0.9	0.8	0.7	0.6	0.5	0.4	0.3	0.2	0.1	1.0	0.9	0.8	0.7	0.6	0.5	0.4	0.3	0.2	0.1	1.0	0.9	0.8	0.7	0.6	0.5	0.4	0.3	0.2	0.1	1.0	0.9	0.8	0.7	0.6	0.5	0.4	0.3	0.2	0.1
0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
0.1	0.1	0.258	0.312	0.462	0.731	1.204	3.957	8.480	21.42	53.01	0.258	0.312	0.462	0.731	1.204	3.957	8.480	21.42	53.01	0.258	0.312	0.462	0.731	1.204	3.957	8.480	21.42	53.01	0.258	0.312	0.462	0.731	1.204	3.957	8.480	21.42	53.01				
0.2	0.2	0.473	0.564	0.816	1.246	1.957	3.154	5.230	13.18	10.69	0.473	0.564	0.816	1.246	1.957	3.154	5.230	13.18	10.69	0.473	0.564	0.816	1.246	1.957	3.154	5.230	13.18	10.69	0.473	0.564	0.816	1.246	1.957	3.154	5.230	13.18	10.69				
0.3	0.3	0.616	0.719	0.989	1.456	2.138	3.116	4.416	5.735	5.791	0.616	0.719	0.989	1.456	2.138	3.116	4.416	5.735	5.791	0.616	0.719	0.989	1.456	2.138	3.116	4.416	5.735	5.791	0.616	0.719	0.989	1.456	2.138	3.116	4.416	5.735	5.791				
0.4	0.4	0.678	0.747	1.035	1.408	1.916	2.517	3.080	3.262	2.540	0.678	0.747	1.035	1.408	1.916	2.517	3.080	3.262	2.540	0.678	0.747	1.035	1.408	1.916	2.517	3.080	3.262	2.540	0.678	0.747	1.035	1.408	1.916	2.517	3.080	3.262	2.540				
0.5	0.5	0.671	0.747	0.939	1.211	1.527	1.820	1.966	1.785	1.164	0.671	0.747	0.939	1.211	1.527	1.820	1.966	1.785	1.164	0.671	0.747	0.939	1.211	1.527	1.820	1.966	1.785	1.164	0.671	0.747	0.939	1.211	1.527	1.820	1.966	1.785	1.164				
0.6	0.6	0.614	0.668	0.796	0.963	1.128	1.232	1.199	0.962	0.541	0.614	0.668	0.796	0.963	1.128	1.232	1.199	0.962	0.541	0.614	0.668	0.796	0.963	1.128	1.232	1.199	0.962	0.541	0.614	0.668	0.796	0.963	1.128	1.232	1.199	0.962	0.541				
0.7	0.7	0.529	0.564	0.638	0.723	0.791	0.797	0.708	0.503	0.284	0.529	0.564	0.638	0.723	0.791	0.797	0.708	0.503	0.284	0.529	0.564	0.638	0.723	0.791	0.797	0.708	0.503	0.284	0.529	0.564	0.638	0.723	0.791	0.797	0.708	0.503	0.284				
0.8	0.8	0.435	0.455	0.490	0.523	0.532	0.495	0.388	0.249	0.066	0.435	0.455	0.490	0.523	0.532	0.495	0.388	0.249	0.066	0.435	0.455	0.490	0.523	0.532	0.495	0.388	0.249	0.066	0.435	0.455	0.490	0.523	0.532	0.495	0.388	0.249	0.066				
0.9	0.9	0.344	0.353	0.363	0.363	0.345	0.294	0.209	0.104	0.008	0.344	0.353	0.363	0.363	0.345	0.294	0.209	0.104	0.008	0.344	0.353	0.363	0.363	0.345	0.294	0.209	0.104	0.008	0.344	0.353	0.363	0.363	0.345	0.294	0.209	0.104	0.008				
1.0	1.0	0.262	0.265	0.261	0.246	0.214	0.163	0.095	0.023	-0.041	0.262	0.265	0.261	0.246	0.214	0.163	0.095	0.023	-0.041	0.262	0.265	0.261	0.246	0.214	0.163	0.095	0.023	-0.041	0.262	0.265	0.261	0.246	0.214	0.163	0.095	0.023	-0.041				
1.25	1.25	0.112	0.111	0.098	0.075	0.045	0.011	-0.022	-0.046	-0.051	0.112	0.111	0.098	0.075	0.045	0.011	-0.022	-0.046	-0.051	0.112	0.111	0.098	0.075	0.045	0.011	-0.022	-0.046	-0.051	0.112	0.111	0.098	0.075	0.045	0.011	-0.022	-0.046	-0.051				
1.5	1.5	0.032	0.032	0.023	0.009	-0.009	-0.026	-0.038	-0.044	-0.036	0.032	0.032	0.023	0.009	-0.009	-0.026	-0.038	-0.044	-0.036	0.032	0.032	0.023	0.009	-0.009	-0.026	-0.038	-0.044	-0.036	0.032	0.032	0.023	0.009	-0.009	-0.026	-0.038	-0.044	-0.036				
1.75	1.75	-0.004	-0.002	-0.006	-0.012	-0.020	-0.026	-0.029	-0.026	-0.017	-0.004	-0.002	-0.006	-0.012	-0.020	-0.026	-0.029	-0.026	-0.017	-0.004	-0.002	-0.006	-0.012	-0.020	-0.026	-0.029	-0.026	-0.017	-0.004	-0.002	-0.006	-0.012	-0.020	-0.026	-0.029	-0.026	-0.017				
2.0	2.0	-0.016	-0.013	-0.013	-0.014	-0.016	-0.017	-0.015	-0.010	-0.002	-0.016	-0.013	-0.013	-0.014	-0.016	-0.017	-0.015	-0.010	-0.002	-0.016	-0.013	-0.013	-0.014	-0.016	-0.017	-0.015	-0.010	-0.002	-0.016	-0.013	-0.013	-0.014	-0.016	-0.017	-0.015	-0.010	-0.002				
2.5	2.5	-0.014	-0.011	-0.008	-0.006	-0.003	-0.000	0.004	0.009	0.015	-0.014	-0.011	-0.008	-0.006	-0.003	-0.000	0.004	0.009	0.015	-0.014	-0.011	-0.008	-0.006	-0.003	-0.000	0.004	0.009	0.015	-0.014	-0.011	-0.008	-0.006	-0.003	-0.000	0.004	0.009	0.015				
3.0	3.0	-0.007	-0.004	-0.001	0.002	0.005	0.008	0.012	0.015	0.019	-0.007	-0.004	-0.001	0.002	0.005	0.008	0.012	0.015	0.019	-0.007	-0.004	-0.001	0.002	0.005	0.008	0.012	0.015	0.019	-0.007	-0.004	-0.001	0.002	0.005	0.008	0.012	0.015	0.019				
3.5	3.5	-0.002	0.001	0.003	0.006	0.008	0.011	0.013	0.017	0.018	-0.002	0.001	0.003	0.006	0.008	0.011	0.013	0.017	0.018	-0.002	0.001	0.003	0.006	0.008	0.011	0.013	0.017	0.018	-0.002	0.001	0.003	0.006	0.008	0.011	0.013	0.017	0.018				
4.0	4.0	0.001	0.003	0.005	0.007	0.009	0.011	0.014	0.016	0.016	0.001	0.003	0.005	0.007	0.009	0.011	0.014	0.016	0.016	0.001	0.003	0.005	0.007	0.009	0.011	0.014	0.016	0.016	0.001	0.003	0.005	0.007	0.009	0.011	0.014	0.016	0.016				
6.0	6.0	0.001	0.003	0.004	0.005	0.006	0.007	0.008	0.009	0.010	0.001	0.003	0.004	0.005	0.006	0.007	0.008	0.009	0.010	0.001	0.003	0.004	0.005	0.006	0.007	0.008	0.009	0.010	0.001	0.003	0.004	0.005	0.006	0.007	0.008	0.009	0.010				
8.0	8.0	0.001	0.001	0.002	0.002	0.003	0.004	0.004	0.005	0.005	0.001	0.001	0.002	0.002	0.003	0.004	0.004	0.005	0.005	0.001	0.001	0.002	0.002	0.003	0.004	0.004	0.005	0.005	0.001	0.001	0.002	0.002	0.003	0.004	0.004	0.005	0.005				
10.0	10.0	0.000	0.001	0.001	0.001	0.002	0.002	0.003	0.003	0.003	0.000	0.001	0.001	0.001	0.002	0.002	0.003	0.003	0.003	0.000	0.001	0.001	0.001	0.002	0.002	0.003	0.003	0.003	0.000	0.001	0.001	0.001	0.002	0.002	0.003	0.003	0.003				

TABLE 2.6
INFLUENCE VALUES FOR SURFACE DISPLACEMENTS
POINT LOAD
 $\rho = I \rho^2 / 2\pi h E$

r/h	v	VERTICAL DISPLACEMENT ρ_z (Taylor, 1962)				
		0	0.2	0.4	0.5	
0.05		37.580	35.921	31.052	27.351	
0.1		17.586	16.728	14.260	13.360	
0.2		7.624	7.162	5.897	4.914	
0.3		4.327	4.016	3.154	2.480	
0.4		2.720	2.478	1.827	1.320	
0.5		1.792	1.599	1.092	0.699	
0.6		1.212	1.048	0.635	0.290	
0.7		0.823	0.690	0.352	0.051	
0.8		0.560	0.450	0.168	-0.079	
0.9		0.373	0.286	0.053	-0.160	
1.0		0.250	0.182	-0.011	-0.183	
1.25		0.080	0.031	-0.085	-0.194	
1.5		0.013	-0.002	-0.077	-0.156	
1.75		-0.007	-0.007	-0.048	-0.123	
2.0		-0.012	-0.011	-0.039	-0.083	
2.5		-0.004	-0.017	-0.025	-0.036	
3.0		-0.003	0.001	-0.008	-0.025	
3.5		-0.003	0.000	-0.004	-0.018	
4.0		-0.001	0.000	-0.003	-0.012	
6.0		-0.000	-0.000	-0.001	-0.002	
8.0		-0.000	-0.000	-0.000	-0.000	
10.0		-0.000	-0.000	-0.000	-0.000	

TABLE 2.7
INFLUENCE VALUES FOR SURFACE DISPLACEMENTS
POINT LOAD
 $\rho = I \rho^2 / 2\pi h E$

r/h	v	RADIAL DISPLACEMENT ρ_r (Taylor, 1962)				
		0	0.2	0.4	0.5	
0.05		19.959	14.362	5.559	-0.041	
0.1		9.948	7.124	2.723	-0.078	
0.2		4.896	3.455	1.250	-0.156	
0.3		3.183	2.184	0.716	-0.225	
0.4		2.308	1.523	0.426	-0.288	
0.5		1.773	1.064	0.232	-0.326	
0.6		1.277	0.824	0.102	-0.376	
0.7		1.000	0.620	0.008	-0.405	
0.8		0.789	0.465	-0.063	-0.420	
0.9		0.627	0.349	-0.111	-0.421	
1.0		0.499	0.259	-0.141	-0.417	
1.25		0.292	0.150	-0.175	-0.380	
1.5		0.167	0.048	-0.163	-0.315	
1.75		0.097	0.012	-0.134	-0.250	
2.0		0.060	0.002	-0.109	-0.195	
2.5		0.027	0.003	-0.070	-0.118	
3.0		0.010	-0.002	-0.038	-0.072	
3.5		0.003	-0.008	-0.022	-0.046	
4.0		0.002	-0.000	-0.014	-0.029	
6.0		0.000	-0.000	-0.002	-0.002	
8.0		0.000	-0.000	-0.000	-0.001	
10.0		0.000	-0.000	-0.000	-0.000	

2.2 Line Loading

In sub-sections 2.2.1 to 2.2.7, only solutions for stress are presented. Displacements due to line loading on or in a semi-infinite mass are only meaningful if evaluated as the displacement of one point relative to another point, both points being located neither at the origin of loading nor at infinity.

2.2.1 INFINITE LINE LOAD ACTING WITHIN AN INFINITE SOLID (Integrated Kelvin problem). (Fig.2.6).

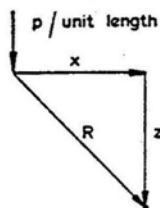


FIG. 2.6

$$\sigma_z = \frac{p}{2\pi(1-\nu)} \frac{z}{R^2} \left[\frac{(3-2\nu)}{2} - \frac{x^2}{R^2} \right] \quad \dots (2.6a)$$

$$\sigma_x = \frac{p}{2\pi(1-\nu)} \frac{z}{R^2} \left[-\frac{(1-2\nu)}{2} + \frac{x^2}{R^2} \right] \quad \dots (2.6b)$$

$$\sigma_y = \frac{p}{2\pi} \frac{\nu}{(1-\nu)} \frac{z}{R^2} \quad \dots (2.6c)$$

$$\tau_{xz} = \frac{p}{2\pi(1-\nu)} \frac{xz}{R^2} \left[\frac{(1-2\nu)}{2} + \frac{z^2}{R^2} \right] \quad \dots (2.6d)$$

2.2.2 INFINITE VERTICAL LINE LOAD ON THE SURFACE OF A SEMI-INFINITE MASS (Integrated Boussinesq problem) (Fig.2.7).

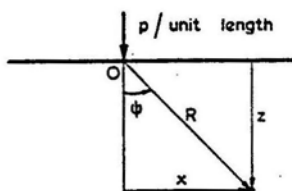


FIG. 2.7

$$\sigma_x = \frac{2p}{\pi} \frac{x^2 z}{R^4} \quad \dots (2.7a)$$

$$\sigma_z = \frac{2p}{\pi} \frac{z^3}{R^4} \quad \dots (2.7b)$$

$$\sigma_y = \frac{2p\nu}{\pi} \frac{z}{R^2} \quad \dots (2.7c)$$

$$\tau_{xz} = \frac{2p}{\pi} \frac{xz^2}{R^4} \quad \dots (2.7d)$$

Principal Stresses:

$$\sigma_1 = \sigma_R = \frac{2p}{\pi} \frac{z}{R^2} \quad \dots (2.8a)$$

$$\sigma_2 = \sigma_\psi = 0 \quad \dots (2.8b)$$

$$\tau_{max} = \frac{p}{\pi} \frac{z}{R^2} \quad \dots (2.8c)$$

Loci of constant σ_1 , σ_2 and τ_{max} are circles tangent to x-axis at 0.

Trajectories of σ_1 are radial lines through 0.

Trajectories of σ_2 are a family of semi-circles, centres at 0.

Trajectories of τ_{max} are two orthogonal families of equi-angular spirals intersecting the radial lines at $\pm 45^\circ$.

For the case of a vertical line load of finite length, influence values for σ_z are tabulated by Lysmer and Duncan (1969).

2.2.3 HORIZONTAL LINE LOAD ACTING ON SURFACE OF SEMI-INFINITE MASS (Integrated Cerruti Problem) (Fig.2.8).

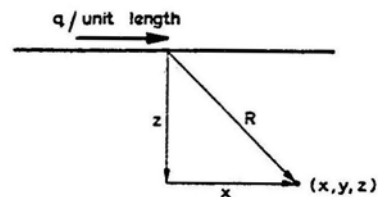


FIG. 2.8

$$\sigma_z = \frac{2qzx^2}{\pi R^4} \quad \dots (2.9a)$$

$$\sigma_x = \frac{2qx^3}{\pi R^4} \quad \dots (2.9b)$$

$$\sigma_y = \frac{2qzx^2}{\pi R^4} \quad \dots (2.9c)$$

$$\tau_{xz} = \frac{2qx^2z}{\pi R^4} \quad \dots (2.9d)$$

$$\tau_{xz} = \frac{px}{\pi} \left[\frac{m+1}{2m} \left\{ \frac{(z-d)^2}{r_1^4} + \frac{z^2-2dz-d^2}{r_2^4} + \frac{8dz(d+z)^2}{r_2^6} \right\} + \frac{m-1}{4m} \left(\frac{1}{r_1^2} - \frac{1}{r_2^2} + \frac{4z(d+z)}{r_2^4} \right) \right] \quad \dots (2.10c)$$

where $m = \frac{1-\nu}{\nu}$.

2.2.5 MELAN'S PROBLEM II

Horizontal line load q/unit length acting beneath the surface of a semi-infinite mass (Fig.2.9).

$$\sigma_z = \frac{qx}{\pi} \left[\frac{m+1}{2m} \left\{ \frac{(z-d)^2}{r_1^4} - \frac{d^2-z^2+6dz}{r_2^4} + \frac{8dzx^2}{r_2^6} \right\} - \frac{m-1}{4m} \left\{ \frac{1}{r_1^2} - \frac{1}{r_2^2} - \frac{4z(d+z)}{r_2^4} \right\} \right] \quad \dots (2.11a)$$

$$\sigma_x = \frac{qx}{\pi} \left[\frac{m+1}{2m} \left\{ \frac{x^2}{r_1^4} + \frac{x^2-4dz-2d^2}{r_2^4} + \frac{8dz(d+z)^2}{r_2^6} \right\} + \frac{m-1}{4m} \left(\frac{1}{r_1^2} + \frac{3}{r_2^2} - \frac{4z(d+z)}{r_2^4} \right) \right] \quad \dots (2.11b)$$

$$\tau_{xz} = \frac{q}{\pi} \left[\frac{m+1}{2m} \left\{ \frac{(z-d)x^2}{r_1^4} + \frac{(2dz+x^2)(d+z)}{r_2^4} - \frac{8dz(d+z)x^2}{r_2^6} \right\} + \frac{m-1}{4m} \left(\frac{z-d}{r_1^2} + \frac{3z+d}{r_2^2} - \frac{4z(d+z)^2}{r_2^4} \right) \right] \quad \dots (2.11c)$$

2.2.6 VERTICAL LINE LOADING ON QUARTER-SPACE (Figure 2.10).

2.2.4 MELAN'S PROBLEM I

Vertical line loading p/unit length beneath the surface of a semi-infinite mass (Fig.2.9).

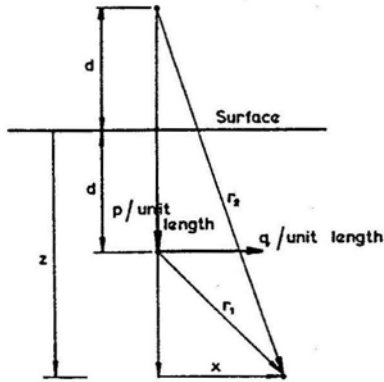


FIG. 2.9

$$\sigma_z = \frac{p}{\pi} \left[\frac{m+1}{2m} \left\{ \frac{(z-d)^3}{r_1^4} + \frac{(z+d) \{ (z+d)^2 + 2dz \}}{r_2^4} - \frac{8dz(d+z)x^2}{r_2^6} \right\} + \frac{m-1}{4m} \left(\frac{z-d}{r_1^2} + \frac{3z+d}{r_2^2} - \frac{4zx^2}{r_2^4} \right) \right] \quad \dots (2.10a)$$

$$\sigma_x = \frac{p}{\pi} \left[\frac{m+1}{2m} \left\{ \frac{(z-d)x^2}{r_1^4} + \frac{(z+d)(x^2+2d^2) - 2dx^2}{r_2^4} + \frac{8dz(d+z)x^2}{r_2^6} \right\} + \frac{m-1}{4m} \left(\frac{z-d}{r_1^2} + \frac{z+3d}{r_2^2} + \frac{4zx^2}{r_2^4} \right) \right] \quad \dots (2.10b)$$

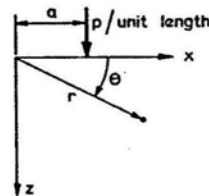


FIG. 2.10

Solutions for the stresses within the quarter-space have been given by Shepherd (1935). Hetenyi (1960) has obtained solutions for the stresses on the boundaries due to both a vertical and a horizontal line load.

Values of stresses obtained by Shepherd are given in Table 2.8. Polar coordinates are used as the problem originally considered was that of an infinite sector.

2.2.7 LINE LOADING AT THE APEX OF AN INFINITE WEDGE (Fig.2.11)

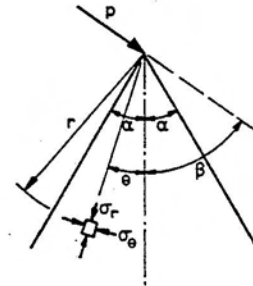


FIG. 2.11

TABLE 2.8
STRESSES IN A QUARTER-SPACE
(After Shepherd, 1935)

Stress	r/a	0.358	0.504	0.710	1.0	1.409	1.985	2.796
θ°								
	0	0	0	0	∞	0	0	0
	15	0.02	0.29	2.63	14.83	1.30	0.09	-0.06
	30	0.13	1.12	4.46	6.59	2.30	0.27	0.01
	45	0.29	1.36	3.03	3.36	1.52	0.34	0.04
$\frac{2\pi a}{p} \sigma_\theta$	60	0.19	0.84	1.56	1.53	0.80	0.21	0.01
	75	0.10	0.23	0.41	0.36	0.22	0.05	-0.06
	90	0	0	0	0	0	0	0
	0	-0.88	-1.26	-1.61	∞	-1.96	-1.90	-1.70
	15	0.04	0.75	1.37	-1.06	3.03	0.63	-0.31
	30	0.22	0.31	-0.42	-0.32	1.93	1.46	0.69
	45	0.04	0.03	-0.24	0.48	1.74	1.85	1.25
$\frac{2\pi a}{p} \sigma_r$	60	0.02	0.09	0.46	1.16	2.09	2.18	1.89
	75	-0.16	0.35	0.45	1.56	2.55	2.79	2.47
	90	-0.64	-0.48	0.23	1.45	2.60	3.13	2.99
	0	0	0	0	0	0	0	0
	15	-0.09	0.26	2.33	-2.49	-2.95	-0.77	-0.35
	30	0.05	0.48	0.70	-2.88	-3.31	-1.45	-0.69
	45	0.02	0.01	-0.90	-2.46	-2.74	-1.59	-0.84
$\frac{2\pi a}{p} \tau_{r\theta}$	60	0.01	-0.38	-1.20	-1.80	-2.01	-1.35	-0.75
	75	-0.01	-0.28	-0.79	-1.19	-1.15	-0.81	-0.43
	90	0	0	0	0	0	0	0

Michell (1900) gives the following solutions:

$$\sigma_r = \frac{2p}{r} \left[\frac{\cos\beta \cos\theta}{2\alpha + \sin 2\alpha} + \frac{\sin\beta \sin\theta}{2\alpha - \sin 2\alpha} \right] \dots (2.12a)$$

$$\sigma_\theta = 0 \dots (2.12b)$$

$$\tau_{r\theta} = 0 \dots (2.12c)$$

2.2.8 VERTICAL LINE LOAD ON FINITE LAYER (Fig.2.12)

Values of stresses and displacements are given in TABLES 2.9 to 2.13 (Poulos, 1966).

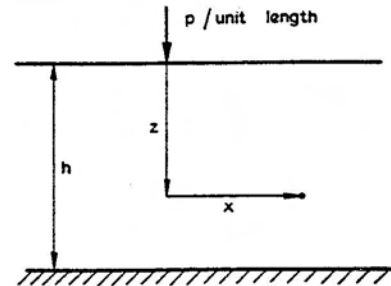


FIG. 2.12

TABLE 2.9
 INFLUENCE VALUES I_{σ_z} FOR VERTICAL STRESS σ_z
 LINE LOAD
 $\sigma_z = \frac{P}{\pi h} I_{\sigma_z}$

$v=0$	$v=0.2$
$v=0.4$	$v=0.5$

$\frac{z/h}{x/h}$	1.0	0.9	0.8	0.7	0.6	0.4	0.2							
0	2.634 2.539	2.566 2.580	2.787 2.787	2.759 2.865	2.980 3.025	2.975 3.113	3.249 3.312	3.256 3.391	3.641 3.704	3.653 3.772	5.157 5.201	5.127 5.234	9.891 9.905	9.899 9.911
0.1	2.573 2.471	2.503 2.508	2.713 2.702	2.682 2.774	2.885 2.920	2.877 3.001	3.118 3.171	3.122 3.243	3.443 3.498	3.452 3.562	4.516 4.585	4.505 4.586	5.946 5.957	5.952 5.960
0.2	2.400 2.291	2.331 2.312	2.505 2.477	2.471 2.528	2.627 2.642	2.614 2.703	2.774 2.810	2.773 2.867	2.948 2.990	2.954 3.040	3.251 3.283	3.217 3.305	2.341 2.351	2.347 2.343
0.3	2.144 2.021	2.075 2.023	2.203 2.145	2.162 2.170	2.261 2.245	2.240 2.279	2.311 2.318	2.302 2.349	2.335 2.352	2.333 2.383	2.099 2.117	2.081 2.128	0.918 0.923	0.922 0.907
0.4	1.840 1.711	1.774 1.689	1.855 1.773	1.810 1.766	1.857 1.810	1.828 1.815	1.830 1.808	1.812 1.815	1.751 1.744	1.741 1.755	1.301 1.307	1.257 1.307	0.407 0.409	0.409 0.389
0.5	1.525 1.389	1.462 1.350	1.504 1.397	1.455 1.365	1.465 1.387	1.429 1.365	1.391 1.338	1.365 1.320	1.265 1.231	1.247 1.223	0.803 0.792	0.774 0.786	0.205 0.201	0.204 0.187
0.6	1.223 1.092	1.168 1.039	1.179 1.061	1.130 1.008	1.117 1.021	1.077 0.978	1.024 0.951	0.993 0.916	0.889 0.837	0.867 0.814	0.497 0.476	0.443 0.466	0.110 0.105	0.107 0.099
0.7	0.954 0.830	0.906 0.771	0.898 0.774	0.850 0.711	0.827 0.718	0.785 0.661	0.733 0.644	0.698 0.594	0.611 0.543	0.584 0.509	0.308 0.276	0.270 0.264	0.062 0.053	0.056 0.058
0.8	0.721 0.615	0.683 0.553	0.661 0.547	0.619 0.478	0.592 0.485	0.553 0.421	0.508 0.416	0.473 0.360	0.408 0.335	0.379 0.294	0.185 0.150	0.124 0.137	0.032 0.023	0.024 0.035
0.9	0.536 0.447	0.507 0.390	0.479 0.375	0.443 0.309	0.417 0.314	0.380 0.251	0.345 0.254	0.311 0.195	0.267 0.192	0.237 0.149	0.108 0.070	0.066 0.057	0.015 0.005	0.006 0.020
1.0	0.357 0.294	0.338 0.241	0.306 0.224	0.279 0.162	0.254 0.167	0.224 0.106	0.199 0.117	0.169 0.062	0.144 0.075	0.116 0.032	0.045 0.010	-0.020 -0.005	-0.000 -0.007	-0.011 0.005
1.25	0.121 0.111	0.123 0.079	0.091 0.059	0.082 0.020	0.062 0.020	0.047 -0.023	0.035 -0.010	0.018 -0.055	0.013 -0.028	-0.005 -0.064	-0.016 -0.037	-0.057 -0.057	-0.014 -0.015	-0.024 -0.021
1.5	0.010 0.043	0.027 0.026	-0.005 0.011	0.003 -0.013	-0.018 -0.014	-0.016 -0.042	-0.028 -0.031	-0.031 -0.064	-0.033 -0.040	-0.040 -0.070	-0.035 -0.035	-0.097 -0.061	-0.019 -0.011	-0.026 -0.034
1.75	-0.030 0.024	-0.006 0.018	-0.036 0.005	-0.019 -0.006	-0.040 -0.010	-0.029 -0.027	-0.042 -0.021	-0.036 -0.046	-0.039 -0.026	-0.038 -0.049	-0.032 -0.021	-0.064 -0.047	-0.017 -0.005	-0.019 -0.034
2.0	-0.042 0.025	-0.014 0.024	-0.042 0.015	-0.019 0.010	-0.041 0.006	-0.022 -0.003	-0.039 -0.001	-0.026 -0.014	-0.035 -0.005	-0.026 -0.020	-0.025 -0.005	-0.131 -0.026	-0.013 0.002	-0.011 -0.025
2.5	-0.025 0.027	-0.003 0.034	-0.022 0.022	-0.004 0.027	-0.019 0.018	-0.004 0.020	-0.016 0.014	-0.004 0.001	-0.013 0.011	-0.004 0.009	-0.006 0.007	-0.042 -0.000	0.000 0.006	0.002 -0.006
3.0	-0.010 0.022	0.004 0.032	-0.007 0.019	0.004 0.028	-0.005 0.017	0.004 0.023	-0.003 0.014	0.004 0.015	-0.000 0.012	0.004 0.015	0.003 0.008	0.005 0.008	0.006 0.005	0.006 0.003
4.0	0.006 0.008	0.005 0.019	0.006 0.006	0.005 0.017	0.007 0.005	0.004 0.015	0.007 0.004	0.005 0.013	0.008 0.003	0.005 0.011	0.009 0.001	0.005 0.008	0.009 0.000	0.004 0.007
6.0	0.005 0.000	0.000 0.006	0.005 -0.000	0.000 0.005	0.005 -0.000	0.000 0.004	0.005 -0.001	0.000 0.004	0.005 -0.001	0.000 0.004	0.005 -0.001	0.000 0.003	0.005 -0.001	0.000 0.002
8.0	0.002 -0.001	-0.000 0.001	0.002 -0.001	-0.000 0.000	0.002 -0.001	-0.001 0.000	0.002 -0.001	-0.001 0.000	0.002 -0.001	-0.001 0.000	0.002 -0.001	-0.001 -0.000	0.002 -0.001	-0.001 -0.001

TABLE 2.10
INFLUENCE VALUES I_{θ} FOR BULK STRESS θ

LINE LOAD

$$\theta = \frac{P}{\pi h} I_{\theta}$$

$\nu=0$	$\nu=0.2$
$\nu=0.4$	$\nu=0.5$

$\frac{z/h}{x/h}$	1.0	0.9	0.8	0.7	0.6	0.4	0.2							
0	2.634 5.923	3.789 7.739	2.546 5.419	3.567 6.933	2.569 5.145	3.508 6.433	2.706 5.120	3.611 6.217	2.986 5.322	3.902 6.295	4.292 6.846	5.419 7.709	8.885 13.02	10.92 14.12
0.1	2.573 5.766	3.707 7.524	2.492 5.294	3.499 6.761	2.512 5.012	3.441 6.291	2.637 4.997	3.530 6.075	2.887 5.162	3.786 6.121	3.975 6.390	5.042 7.217	6.647 9.880	8.230 10.76
0.2	2.400 5.346	3.436 6.936	2.338 4.960	3.267 6.318	2.352 4.726	3.214 5.903	2.444 4.673	3.272 5.683	2.620 4.755	3.445 5.651	3.253 5.371	4.165 6.116	3.759 5.848	4.764 6.432
0.3	2.144 4.716	3.063 6.068	2.107 4.447	2.947 5.632	2.115 4.216	2.900 5.315	2.166 4.188	2.920 5.106	2.256 4.176	2.997 4.997	2.458 4.226	3.206 4.875	1.961 3.326	2.605 3.737
0.4	1.840 3.992	2.559 5.068	1.829 3.847	2.538 4.840	1.832 3.718	2.504 4.620	1.847 3.635	2.492 4.434	1.862 3.553	2.490 4.272	1.770 3.239	2.363 3.803	0.970 1.952	1.413 2.275
0.5	1.525 3.240	2.142 4.050	1.536 3.206	2.129 3.992	1.536 3.112	2.107 3.882	1.524 3.057	2.075 3.739	1.489 2.937	2.021 3.565	1.239 2.452	1.716 2.947	0.423 1.183	0.754 1.476
0.6	1.223 2.548	1.685 3.117	1.249 2.599	1.709 3.203	1.249 2.568	1.701 3.175	1.221 2.517	1.662 3.079	1.157 2.390	1.583 2.919	0.844 1.868	1.220 2.308	0.107 0.754	0.370 1.045
0.7	0.954 1.937	1.302 2.314	0.989 2.045	1.349 2.484	0.989 2.037	1.352 2.533	0.955 2.026	1.313 2.486	0.879 1.910	1.228 2.362	0.559 1.423	0.867 1.820	-0.070 0.499	0.153 0.808
0.8	0.721 1.436	0.949 1.661	0.759 1.575	1.008 1.890	0.760 1.612	1.019 1.979	0.724 1.608	0.987 1.973	0.647 1.514	0.909 1.887	0.350 1.100	0.590 1.462	-0.175 0.360	0.020 0.679
0.9	0.537 1.044	0.696 1.171	0.574 1.195	0.756 1.409	0.575 1.230	0.773 1.533	0.541 1.263	0.747 1.560	0.470 1.194	0.679 1.514	0.207 0.858	0.409 1.191	-0.224 0.279	-0.044 0.610
1.0	0.357 0.686	0.394 0.724	0.390 0.837	0.451 0.972	0.392 0.901	0.472 1.105	0.362 0.936	0.457 1.160	0.300 0.895	0.409 1.150	0.080 0.651	0.213 0.956	-0.258 0.234	-0.098 0.565
1.25	0.121 0.258	0.079 0.238	0.142 0.373	0.112 0.417	0.142 0.411	0.128 0.547	0.122 0.484	0.127 0.619	0.082 0.483	0.109 0.666	-0.052 0.389	0.029 0.634	-0.242 0.212	-0.090 0.514
1.5	0.010 0.101	-0.014 0.079	0.020 0.176	0.002 0.229	0.018 0.214	0.011 0.299	0.004 0.268	0.013 0.353	-0.020 0.285	0.009 0.405	-0.097 0.272	-0.017 0.475	-0.200 0.222	-0.056 0.465
1.75	-0.030 0.056	-0.021 0.055	-0.027 0.102	-0.015 0.141	-0.030 0.142	-0.011 0.200	-0.039 0.170	-0.009 0.252	-0.053 0.191	-0.009 0.299	-0.095 0.211	-0.014 0.376	-0.148 0.217	-0.020 0.419
2.0	-0.042 0.058	-0.022 0.072	-0.043 0.084	-0.018 0.118	-0.047 0.110	-0.015 0.163	-0.052 0.130	-0.013 0.205	-0.060 0.149	-0.012 0.243	-0.081 0.179	-0.013 0.311	-0.105 0.206	-0.013 0.368
2.5	-0.025 0.062	-0.005 0.102	-0.027 0.073	-0.004 0.124	-0.030 0.086	-0.003 0.147	-0.033 0.097	-0.001 0.170	-0.036 0.108	-0.000 0.193	-0.041 0.131	0.002 0.238	-0.045 0.152	0.005 0.281
3.0	-0.010 0.052	0.005 0.097	-0.012 0.058	0.005 0.111	-0.013 0.064	0.005 0.125	-0.015 0.073	0.006 0.139	-0.016 0.079	0.007 0.154	-0.017 0.093	0.009 0.182	-0.019 0.105	0.013 0.208
4.0	0.006 0.018	0.007 0.058	0.005 0.021	0.007 0.064	0.005 0.026	0.007 0.070	0.004 0.027	0.007 0.075	0.004 0.029	0.007 0.081	0.004 0.034	0.007 0.091	-0.000 0.037	0.008 0.101
6.0	0.005 0.000	0.001 0.017	0.005 0.001	0.001 0.018	0.005 0.001	0.001 0.020	0.005 0.002	0.001 0.021	0.005 0.003	0.001 0.022	0.005 0.004	0.001 0.025	0.003 0.004	0.000 0.027
8.0	0.002 -0.002	-0.001 0.002	0.002 -0.002	-0.001 0.002	0.002 -0.002	-0.001 0.003	0.002 -0.001	-0.001 0.003	0.002 -0.001	-0.001 0.003	0.002 -0.001	-0.001 0.004	0.002 -0.001	-0.001 0.004

TABLE 2.11
INFLUENCE VALUES $I_{\tau_{xz}}$ FOR HORIZONTAL SHEAR STRESS τ_{xz}
LINE LOAD

$$\tau_{xz} = \frac{P}{\pi h} I_{\tau_{xz}}$$

$v=0$	$v=0.2$
$v=0.4$	$v=0.5$

z/h x/h	1.0	0.9	0.8	0.7	0.6	0.4	0.2
0	0 0	0 0	0 0	0 0	0 0	0 0	0 0
0.1	0.143 0.266	0.195 0.330	0.164 0.223	0.193 0.250	0.219 0.242	0.234 0.248	0.309 0.310
0.2	0.265 0.496	0.364 0.617	0.300 0.416	0.357 0.468	0.391 0.439	0.422 0.453	0.536 0.542
0.3	0.348 0.666	0.485 0.830	0.388 0.554	0.470 0.630	0.490 0.563	0.536 0.588	0.643 0.657
0.4	0.391 0.766	0.554 0.960	0.427 0.635	0.529 0.732	0.517 0.616	0.578 0.655	0.645 0.671
0.5	0.391 0.799	0.572 1.007	0.421 0.660	0.539 0.775	0.489 0.610	0.561 0.666	0.576 0.617
0.6	0.367 0.780	0.552 0.993	0.385 0.644	0.514 0.774	0.427 0.571	0.510 0.643	0.476 0.534
0.7	0.319 0.724	0.506 0.933	0.332 0.600	0.465 0.740	0.353 0.511	0.442 0.599	0.370 0.444
0.8	0.268 0.649	0.446 0.849	0.271 0.540	0.407 0.688	0.273 0.447	0.372 0.550	0.275 0.363
0.9	0.212 0.567	0.383 0.757	0.215 0.475	0.348 0.627	0.210 0.385	0.307 0.500	0.197 0.295
1.0	0.152 0.460	0.303 0.636	0.148 0.392	0.274 0.544	0.139 0.312	0.234 0.438	0.119 0.226
1.25	0.055 0.285	0.177 0.440	0.054 0.252	0.161 0.401	0.046 0.199	0.131 0.338	0.030 0.136
1.5	0.012 0.170	0.098 0.311	0.004 0.157	0.090 0.300	0.003 0.127	0.073 0.268	-0.004 0.087
1.75	-0.014 0.099	0.053 0.228	-0.015 0.094	0.051 0.228	-0.011 0.079	0.042 0.213	-0.010 0.055
2.0	0.011 0.054	0.029 0.172	-0.018 0.053	0.029 0.175	-0.010 0.045	0.026 0.169	-0.004 0.033
2.5	0.002 0.016	0.017 0.116	-0.005 0.015	0.018 0.118	0.005 0.011	0.018 0.114	0.015 0.109
3.0	-0.004 -0.000	0.015 0.087	0.005 -0.003	0.015 0.087	0.015 -0.006	0.015 0.084	0.026 -0.011
4.0	0.007 -0.020	0.012 0.045	0.015 -0.022	0.012 0.043	0.023 -0.025	0.011 0.040	0.030 -0.028
6.0	0.007 -0.026	0.007 0.015	0.012 -0.028	0.006 0.014	0.017 -0.029	0.005 0.013	0.022 -0.030
8.0	0.004 -0.028	0.004 0.006	0.007 -0.024	0.003 0.005	0.011 -0.025	0.003 0.005	0.014 -0.026

TABLE 2.12

INFLUENCE VALUES $I_{\rho_{xz}}$ FOR SURFACE DISPLACEMENTS

LINE LOAD

$$\rho_{xz} = \frac{p}{\pi E} I_{\rho_{xz}}$$

HORIZONTAL DISPLACEMENT ρ_{xz}

x/h	ν	0	0.2	0.4	0.5
0.1		1.366	0.952	0.316	-0.094
0.2		1.246	0.834	0.217	-0.184
0.3		1.130	0.727	0.127	-0.269
0.4		1.019	0.613	0.042	-0.342
0.5		0.872	0.518	-0.033	-0.407
0.6		0.749	0.436	-0.097	-0.460
0.7		0.646	0.353	-0.151	-0.499
0.8		0.556	0.288	-0.194	-0.524
0.9		0.476	0.227	-0.223	-0.538
1.0		0.390	0.180	-0.247	-0.537
1.25		0.257	0.084	-0.261	-0.509
1.5		0.167	0.026	-0.241	-0.448
1.75		0.111	0.005	-0.211	-0.384
2.0		0.074	0.001	-0.175	-0.308
2.5		0.034	-0.002	-0.114	-0.210
3.0		0.012	-0.015	-0.067	-0.135
4.0		0.002	-0.002	-0.026	-0.053
6.0		0.000	0.000	-0.005	-0.007
8.0		0.000	0.000	-0.000	-0.000

TABLE 2.13

INFLUENCE VALUES I_{ρ_z} FOR SURFACE DISPLACEMENTS

LINE LOAD

$$\rho_z = \frac{p}{\pi E} I_{\rho_z}$$

VERTICAL DISPLACEMENT ρ_z

x/h	ν	0	0.2	0.4	0.5
0.1		3.756	3.466	2.633	1.926
0.2		2.461	2.222	1.558	0.973
0.3		1.730	1.533	0.964	0.458
0.4		1.244	1.069	0.583	0.132
0.5		0.896	0.749	0.324	-0.079
0.6		0.643	0.511	0.145	-0.217
0.7		0.453	0.347	0.045	-0.299
0.8		0.313	0.218	-0.057	-0.344
0.9		0.212	0.141	-0.101	-0.358
1.0		0.126	0.059	-0.139	-0.359
1.25		0.023	-0.006	-0.146	-0.315
1.5		-0.012	-0.024	-0.112	-0.254
1.75		-0.023	-0.023	-0.086	-0.198
2.0		-0.017	-0.015	-0.071	-0.134
2.5		-0.010	-0.009	-0.034	-0.085
3.0		-0.008	-0.001	-0.014	-0.057
4.0		-0.002	-0.000	-0.007	-0.025
6.0		-0.000	-0.000	-0.002	-0.004
8.0		-0.000	-0.000	-0.000	-0.000

2.3 Line Loading—Axial Symmetry

2.3.1 UNIFORM VERTICAL RING LOADING ON SURFACE OF SEMI-INFINITE MASS (Fig. 2.13)

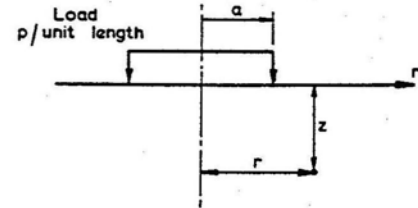


FIG. 2.13

On the axis ($r=0$),

$$\sigma_z = \frac{3pz^3 a}{(a^2+z^2)^{5/2}} \quad \dots (2.13a)$$

$$\sigma_r = \sigma_\theta = \frac{pza}{2(a^2+z^2)^{5/2}} [2(1+\nu)(a^2+z^2) - 3z^2] \quad \dots (2.13b)$$

$$\theta = \frac{2p(1+\nu)za}{(a^2+z^2)^{3/2}} \quad \dots (2.13c)$$

$$\tau_{rz} = 0 \quad \dots (2.13d)$$

$$\rho_z = \frac{p(1+\nu)a}{E(a^2+z^2)^{3/2}} \left[2(1-\nu) + \frac{z^2}{(a^2+z^2)} \right] \quad \dots (2.13e)$$

$$\rho_r = 0 \quad \dots (2.13f)$$

2.3.2 UNIFORM VERTICAL SUBSURFACE LINE LOAD (Fig. 2.14)

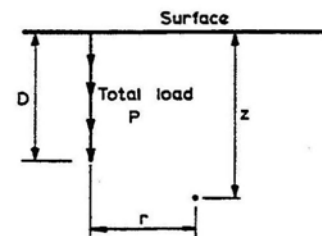


FIG. 2.14

Geddes (1966) evaluated the following expressions for the stresses from Mindlin's equations:

$$K_{zz} = \sigma_z \frac{D^2}{P} = \frac{-1}{8\pi(1-\nu)} \left[-\frac{2(2-\nu)}{A} + \frac{2(2-\nu) + 2(1-2\nu) \frac{m}{n} (\frac{m}{n} + \frac{1}{n})}{B} - \frac{(1-2\nu)2 (\frac{m}{n})^2}{F} + \frac{n^2}{A^3} + \frac{4m^2-4(1+\nu)(\frac{m}{n})^2 m^2}{F^3} + \frac{4m(1+\nu)(m+1)(\frac{m}{n} + \frac{1}{n})^2 - (4m^2+n^2)}{B^3} + \frac{6m^2(\frac{m^4-n^4}{n^2})}{F^5} + \frac{6m(mn^2 - \frac{1}{n^2}[m+1]^5)}{B^5} \right] \dots (2.14a)$$

$$K_{rr} = \sigma_r \frac{D^2}{P} = \frac{-1}{8\pi(1-\nu)} \left[\frac{(1-2\nu)}{A} + \frac{(7-2\nu)-12(1-\nu) \frac{m}{n} (\frac{m}{n} + \frac{1}{n})}{B} - \left\{ \frac{4(2-\nu)-12(1-\nu)(\frac{m}{n})^2}{F} \right\} - \frac{n^2}{A^3} + \frac{4n^2-2m^2+2(1+2\nu)(\frac{m}{n})^2 m^2}{F^3} - \left\{ \frac{3n^2-2m^2+2(1+2\nu)\frac{m}{n}(m+1)^2(\frac{m}{n} + \frac{1}{n})}{B^3} \right\} + \frac{6[n^2m^2-m^4(\frac{m}{n})^2]}{F^5} + \frac{6[\frac{m}{n}(m+1)^4(\frac{m}{n} + \frac{1}{n}) - m^2n^2]}{B^5} + 4(1-\nu)(1-2\nu) \left\{ \frac{1}{F+m} - \frac{1}{B+m+1} \right\} \right] \dots (2.14b)$$

$$K_{\theta\theta} = \sigma_\theta \frac{D^2}{P} = \frac{-1}{8\pi(1-\nu)} \left[\frac{(1-2\nu)}{A} + \frac{6-(1-2\nu)(3-4\nu)+6(1-2\nu)\frac{m}{n}(\frac{m}{n} + \frac{1}{n})}{B} + \frac{2(1-2\nu)^2-6(1-2\nu)(\frac{m}{n})^2-6}{F} + \frac{2m^2-4\nu n^2+2(1+2\nu)\frac{m}{n}(m+1)^2(\frac{m}{n} + \frac{1}{n})}{B^3} \right]$$

$$+ \frac{4\nu n^2-2m^2-2(1+2\nu)m^2(\frac{m}{n})^2}{F^3} - 4(1-\nu)(1-2\nu) \left\{ \frac{1}{F+m} - \frac{1}{B+m+1} \right\} \dots (2.14c)$$

$$K_{rz} = \tau_{rz} \frac{D^2}{P} = \frac{-n}{8\pi(1-\nu)} \left[\frac{(1-2\nu)\frac{1}{n}(\frac{m}{n} - \frac{1}{n})}{A} + \frac{(m-1)(\frac{m}{n} - \frac{1}{n})^2}{A^3} + \frac{(1-2\nu)\frac{1}{n}(\frac{m}{n} + \frac{1}{n})\frac{6}{n}(\frac{m}{n})^2(\frac{m}{n} + \frac{1}{n})}{B} + \frac{12m-4\nu m+(m+1)^3 \left[(\frac{1}{n})^2 + 12 \frac{m^2}{n^4} \right]}{B^3} - \frac{6(\frac{m}{n})^2(m+1)^3(\frac{m}{n} + \frac{1}{n})^2 + 6mn^2}{B^5} + \frac{6(\frac{m}{n})^3(\frac{1}{n}) - 2(1-2\nu)\frac{m}{n^2}}{F} + \frac{4\nu m-12m-m^3 \left[\frac{2}{n^2} + 12 \frac{m^2}{n^4} \right] + 6mn^2+6m^3(\frac{m}{n})^4}{F^3 + F^5} \right] \dots (2.14d)$$

On the axis with $n=0$ and $m>1.0$,

$$K_{zz} = \frac{-1}{8\pi(1-\nu)} \left[-\frac{4(1-\nu)}{m} - \frac{2(2-\nu)}{(m-1)} + \frac{2(2-\nu)}{(m+1)} + \frac{4m(2-\nu)}{(m+1)^2} - \frac{4m^2}{(m+1)^3} \right] \dots (2.14e)$$

$$K_{rr} = K_{\theta\theta} = \frac{-1}{8\pi(1-\nu)} \left[-\frac{2+2\nu(1-2\nu)}{m} + \frac{(1-2\nu)}{(m-1)} + \frac{6-(1-2\nu)^2}{(m+1)} - \frac{6m}{(m+1)^2} + \frac{2m^2}{(m+1)^3} \right] \dots (2.14f)$$

$$K_{rz} = 0 \dots (2.14g)$$

where $n = r/D$, $m = z/D$
 $F^2 = n^2 + m^2$
 $A^2 = [n^2 + (m-1)^2]$
 $B^2 = [n^2 + (m+1)^2]$.

K_{zz} throughout the mass, and K_{rr} and $K_{\theta\theta}$ on the axis, are tabulated by Geddes (1966).

2.3.3 LINEARLY VARYING SUBSURFACE LINE LOAD
(Fig. 2.15)

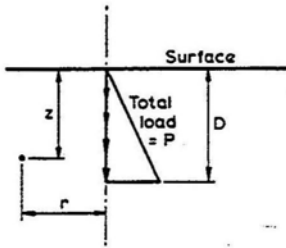


FIG. 2.15

The following expressions for stress have been obtained by Geddes (1966):

$$\begin{aligned}
 K_{zz} = \sigma_z \frac{D^2}{P} &= \frac{-1}{4\pi(1-\nu)} \left[\frac{-2(2-\nu)}{A} \right. \\
 &+ \frac{2(2-\nu)(4m+1) - 2(1-2\nu) \left(\frac{m}{n}\right)^2 (m+1)}{B} \\
 &+ \frac{2(1-2\nu) \frac{m^3}{n^2} - 8(2-\nu)m}{F} + \frac{mm^2 + (m-1)^3}{A^3} \\
 &+ \frac{4\nu n^2 m + 4m^3 - 15n^2 m - 2(5+2\nu) \left(\frac{m}{n}\right)^2 (m+1)^3 + (m+1)^3}{B^3} \\
 &+ \frac{2(7-2\nu)mn^2 - 6m^3 + 2(5+2\nu) \left(\frac{m}{n}\right)^2 m^3}{F^3} \\
 &+ \frac{6mn^2(n^2 - m^2) + 12 \left(\frac{m}{n}\right)^2 (m+1)^5}{B^5} \\
 &- \frac{12 \left(\frac{m}{n}\right)^2 m^5 + 6mn^2(n^2 - m^2)}{F^5} \\
 &\left. - 2(2-\nu) \log_e \left(\frac{A+m-1}{F+m} \cdot \frac{B+m+1}{F+m} \right) \right] \quad \dots (2.15a)
 \end{aligned}$$

$$\begin{aligned}
 K_{rr} = \sigma_r \frac{D^2}{P} &= \frac{-1}{4\pi(1-\nu)} \left[\frac{(1-2\nu)}{A} \right. \\
 &+ \frac{(7-2\nu) - 12m + 12(1-\nu) \left(\frac{m}{n}\right)^2 (m+1)}{B} \\
 &+ \frac{12m - 12(1-\nu) \frac{m^3}{n^2}}{F} - \frac{(m-1)^3 + mm^2}{A^3} \\
 &+ \frac{3(m+1)^3 - 2m^3 + (21-4\nu)mn^2 + 2(5+2\nu) \left(\frac{m}{n}\right)^2 (m+1)^3}{B^3}
 \end{aligned}$$

and,

$$\begin{aligned}
 K_{rz} = \tau_{rz} \frac{D^2}{P} &= \frac{-n}{4\pi(1-\nu)} \left[\frac{2(2-\nu) + (1-2\nu) \frac{m}{n^2} (m-1)}{A} \right. \\
 &- \frac{6 \left(\frac{m}{n}\right)^4}{F} - \frac{2(2-\nu) + (1-2\nu) \frac{m}{n^2} (m+1) - 6 \frac{m^3}{n^4} (m+1)}{B} \\
 &+ \frac{\frac{m}{n} (m-1)^3 - n^2}{A^3}
 \end{aligned}$$

$$\begin{aligned}
 &- \frac{2(5+2\nu) \frac{m^5}{n^2} + 4(5-\nu)mn^2}{F^3} \\
 &+ \frac{6mn^2(m^2 - n^2) - 12 \left(\frac{m}{n}\right)^2 (m+1)^5}{B^5} \\
 &- \frac{6mn^2(m^2 - n^2) - 12 \frac{m^7}{n^2}}{F^5} + (1-2\nu) \log_e \frac{A+m-1}{F+m} \\
 &+ \{(1-2\nu)^2 - 6\} \log \left(\frac{B+m+1}{F+m} \right) \\
 &+ 2(1-\nu)(1-2\nu) \left\{ \frac{m-1}{B+m+1} - \frac{m}{F+m} \right\} \quad \dots (2.15b)
 \end{aligned}$$

$$\begin{aligned}
 K_{\theta\theta} = \sigma_{\theta} \frac{D^2}{P} &= \frac{-1}{4\pi(1-\nu)} \left[\frac{(1-2\nu)}{A} \right. \\
 &- \frac{(1-2\nu)(3-4\nu) + 6(1-2\nu) \left(\frac{m}{n}\right)^2 (m+1) + 6(2m-1)}{B} \\
 &+ \frac{6(1-2\nu) \frac{m^3}{n^2} + 12m}{F} \\
 &- (1-2\nu) \left\{ \frac{2(m+1)^3 + 4mn^2 - 2 \left(\frac{m}{n}\right)^2 (m+1)^3}{B^3} \right\} \\
 &+ \frac{2(m+1)^3 + 6mn^2 - 2n^3 - 6 \left(\frac{m}{n}\right)^2 (m+1)^3}{B^3} \\
 &+ \frac{(2m^3 + 4mn^2 - 2 \frac{m^5}{n^2})(1-2\nu)}{F^3} - \frac{6mn^2 - 6 \frac{m^5}{n^2}}{F^3} \\
 &+ (1-2\nu) \log_e \left(\frac{A+m-1}{F+m} \right) + \{(1-2\nu)^2 - 6\} \\
 &\cdot \log_e \left(\frac{B+m+1}{F+m} \right) - 2(1-\nu)(1-2\nu) \left\{ \frac{m-1}{B+m+1} - \frac{m}{F+m} \right\} \quad \dots (2.15c)
 \end{aligned}$$

$$\begin{aligned}
 & - \frac{(3-4\nu) \frac{m}{n^2} (m+1)^3 + m^2 + 17m^2 - n^2 + 12 \frac{m^3}{n^4} (m+1)^3}{B^3} \\
 & + \frac{2(1-2\nu) \frac{m^4}{n^2} + 4(5-\nu)m^2 + 12 \frac{m^6}{n^4}}{F^3} \\
 & + \frac{6 \frac{m^3}{n^4} (m+1)^5 - 6 \frac{m}{n^2} (m+1)^5 + 12m^2n^2}{B^5} \\
 & - \frac{6 \frac{m^8}{n^4} - 6 \frac{m^6}{n^2} + 12m^2n^2}{F^5}] \\
 & \dots (2.15d)
 \end{aligned}$$

On the loading axis with $m > 1.0$

$$\begin{aligned}
 K_{zz} &= \frac{-1}{4\pi(1-\nu)} \left[2 - \frac{2(2-\nu)m}{(m-1)} + \frac{6(2-\nu)m}{(m+1)} - \frac{2(7-2\nu)m^2}{(m+1)^2} \right. \\
 & \left. + \frac{4m^3}{(m+1)^3} - 2(2-\nu) \log_e \left\{ \frac{m^2-1}{m^2} \right\} \right] \\
 & \dots (2.15e)
 \end{aligned}$$

$$\begin{aligned}
 K_{rr} &= K_{\theta\theta} = \frac{1}{4\pi(1-\nu)} \left[11 - 2(1-2\nu)(1-\nu) + (1-2\nu) \right. \\
 & \cdot \log_e \left\{ \frac{m-1}{m} \right\} + (1-2\nu)^2 \log_e \left\{ \frac{m+1}{m} \right\} - 6 \log_e \left\{ \frac{m+1}{m} \right\} \\
 & + (1-2\nu) \frac{m}{(m-1)} + [(1-2\nu)^2 - 18] \frac{m}{(m+1)} \\
 & \left. + \frac{9m^2}{(m+1)^2} - \frac{2m^3}{(m+1)^3} \right] \\
 & \dots (2.15f)
 \end{aligned}$$

$$K_{rz} = 0 \quad \dots (2.15g)$$

$$\begin{aligned}
 \text{where } n &= r/D, \quad m = z/D \\
 F^2 &= n^2 + m^2 \\
 A^2 &= [n^2 + (m-1)^2] \\
 B^2 &= [n^2 + (m+1)^2]
 \end{aligned}$$

Values of K_{zz} throughout the mass, and K_{rr} and $K_{\theta\theta}$ on the axis, are tabulated by Geddes (1966).