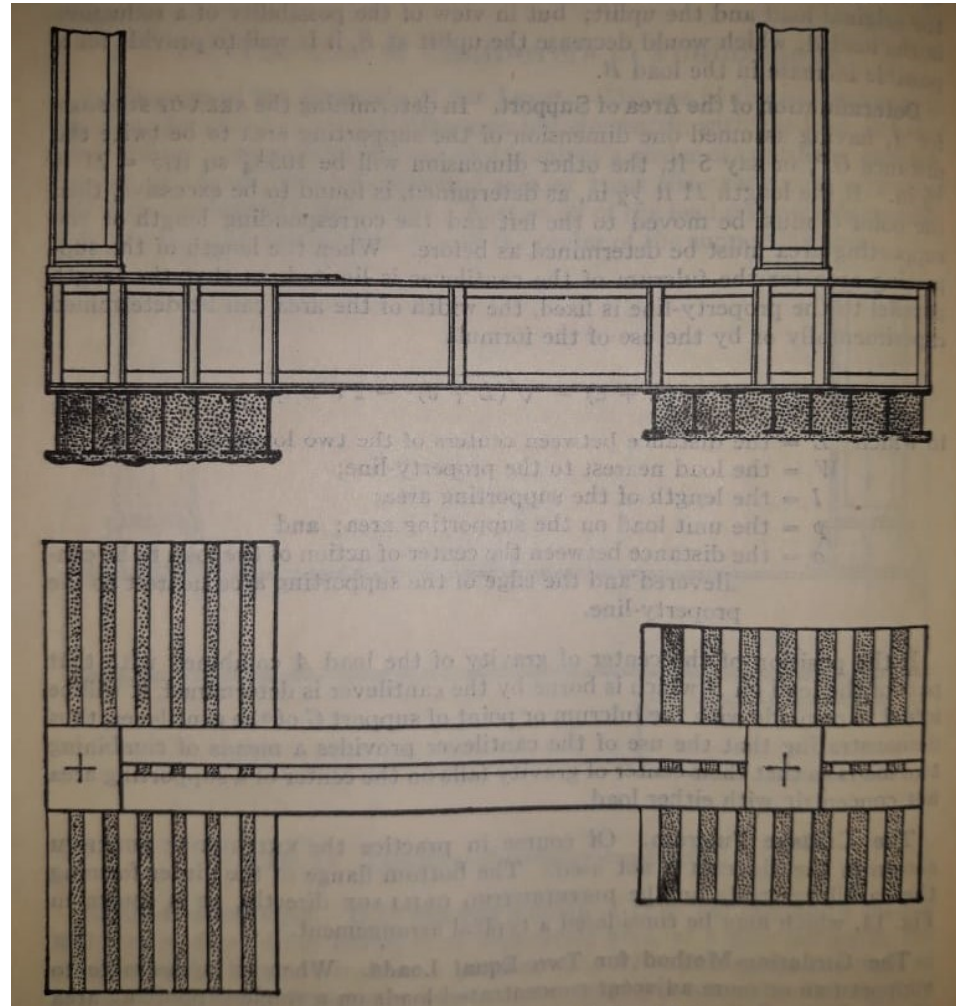


PEF 3405

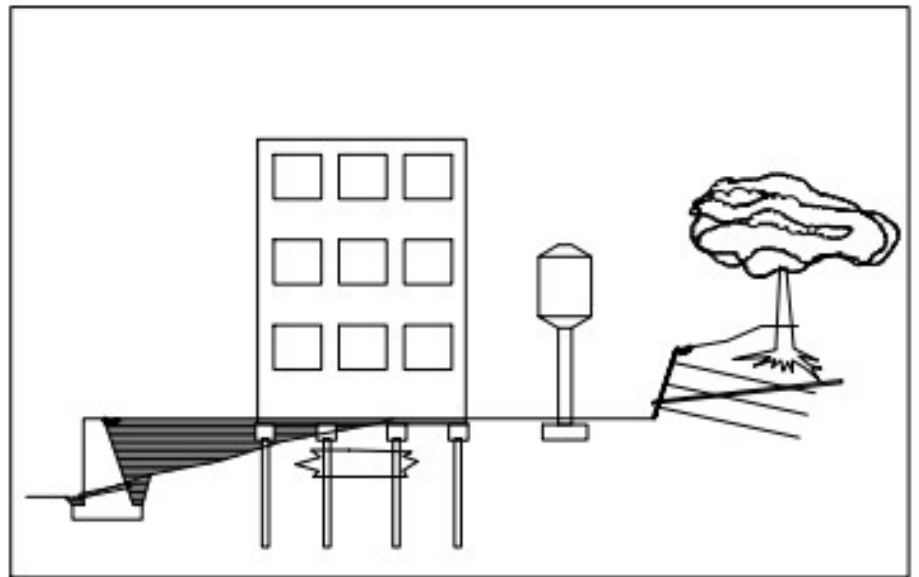
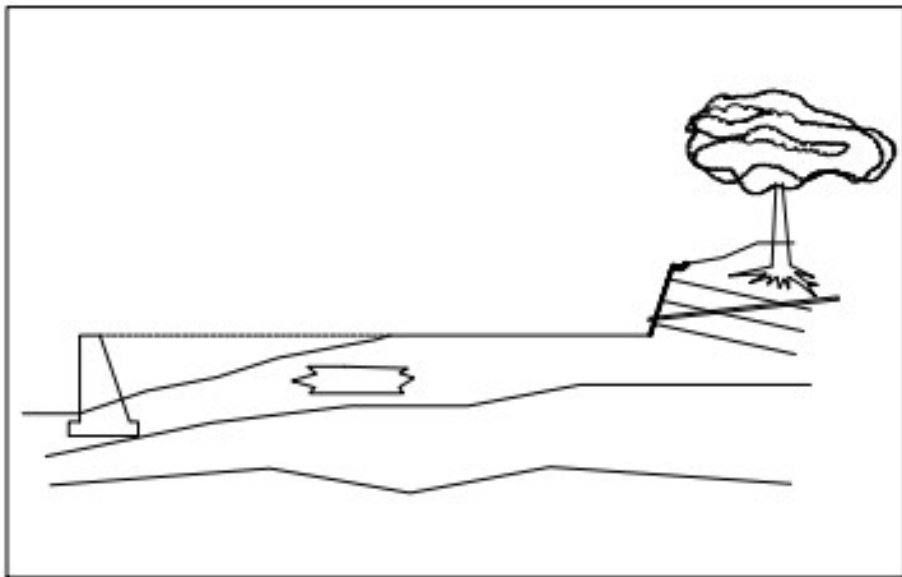
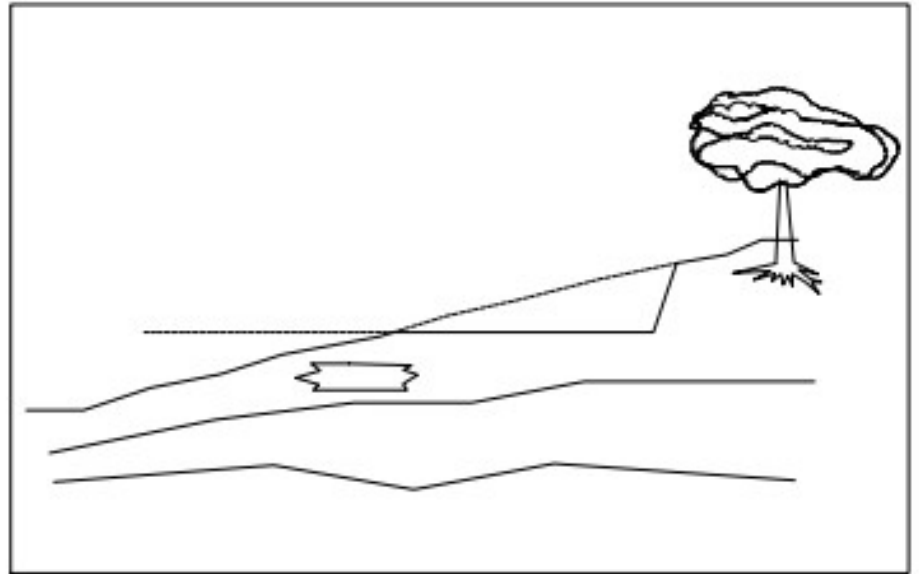
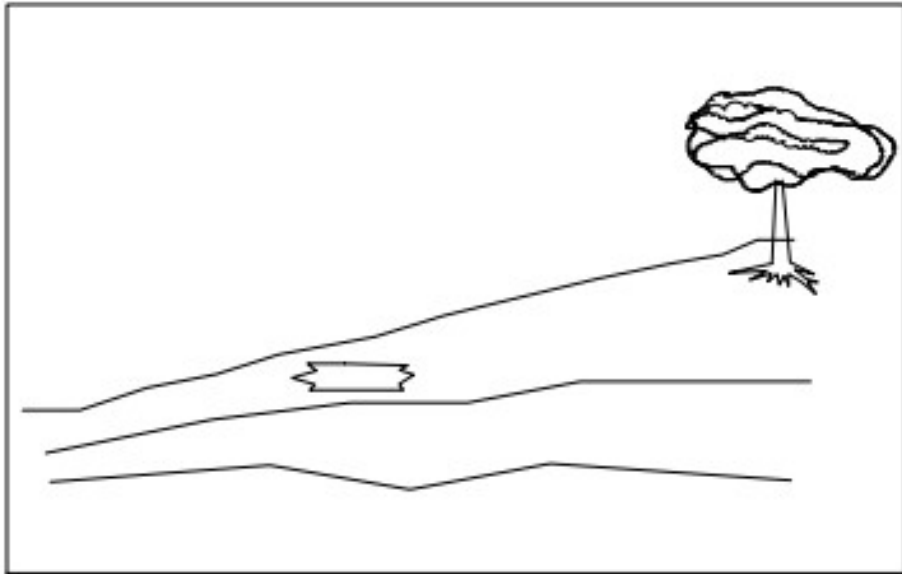
Conceitos iniciais - fundações rasas



Sapatas com viga alavanca – solução com uso de vigas mistas de aço e concreto (Thomas Nolan. T. The use of cantilevers in foundations. Kidder-Parker architects and builders handbook. Wiley & Sons, 1949. 18a Ed. Cap. I, p. 170.)

FUNDAÇÕES E CONTENÇÕES

- FUNDAÇÕES
 - Rasas (ou diretas);
 - Profundas;
 - Especiais;
- CONTENÇÕES
 - Aterros;
 - Escavações;
 - Taludes naturais;



FUNDAÇÕES RASAS

- Algumas formas usuais;
 - Sapatas isoladas e blocos;
 - Sapatas corridas;
 - Sapatas associadas;
 - Sapatas excêntricas com viga alavanca (ou viga de equilíbrio);
 - Radier;
- Transferência dos esforços para o maciço;
 - Tensões na interface entre o elemento de fundação e o solo;
 - Tensões no interior do maciço;
 - Deformações (recalques);

DESLOCAMENTOS NA SUPERFÍCIE

Teoria da Elasticidade (aplicação, com simplificações, ao caso de fundação do tipo sapata)

$$\rho = \sigma \times B \times \frac{1 - \nu^2}{E} \times I_\rho$$

$$I_\rho = \frac{1}{\pi} \times \left[\ell \times \ln \frac{1 + \sqrt{\ell^2 + 1}}{\ell} + \ln \left(\ell + \sqrt{\ell^2 + 1} \right) \right]$$

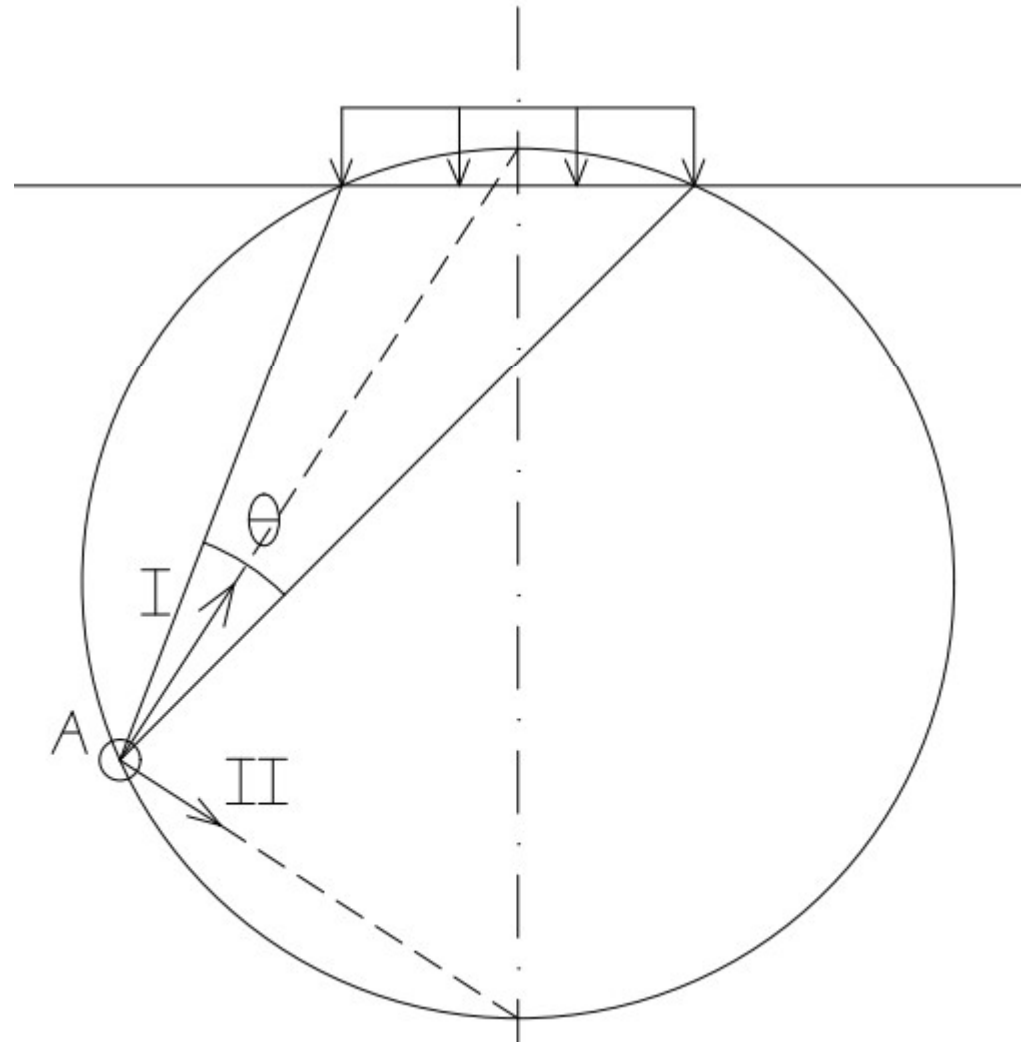
$$\ell = \frac{L}{B}$$

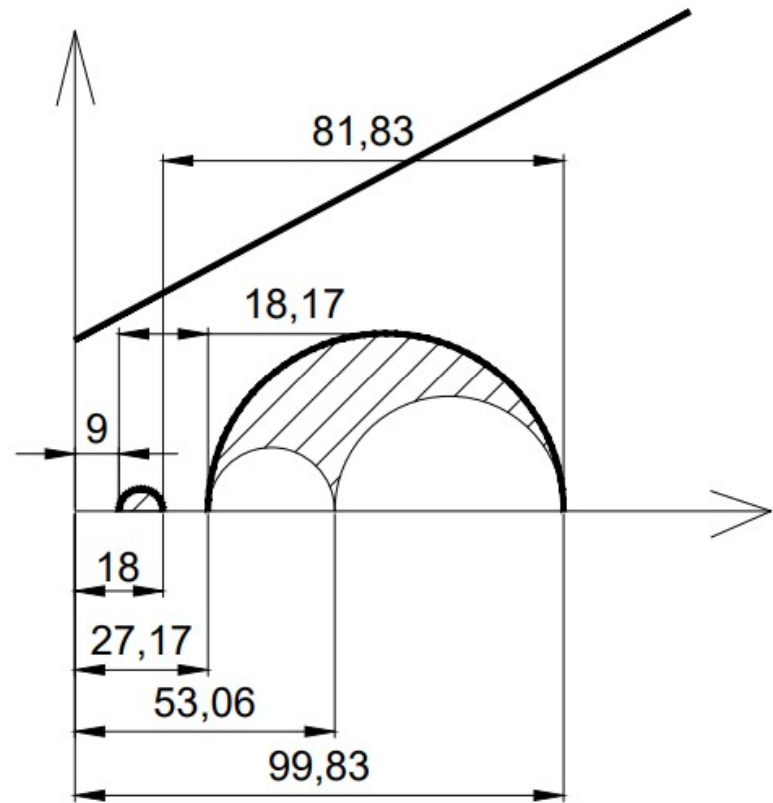
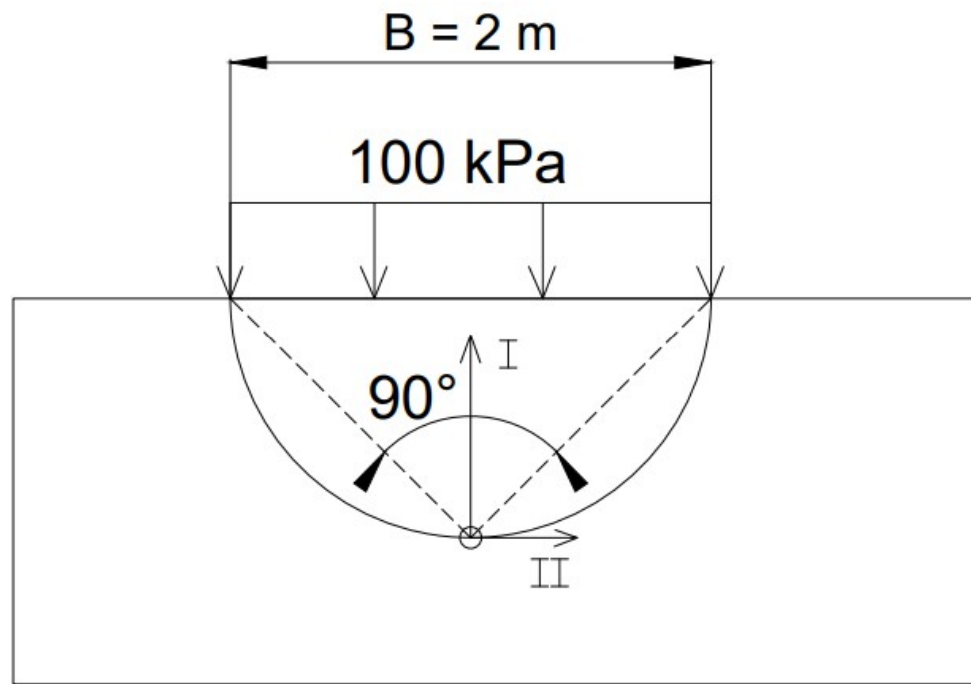
TENSÕES NO INTERIOR DO MACIÇO

Teoria da Elasticidade (aplicação, com simplificação, ao caso de fundação corrida)

$$\sigma_{I,II} = \frac{\sigma_o}{\pi} \times (\theta \pm \sin \theta)$$

$$\tau_{m\acute{a}x} = \frac{\sigma_o}{\pi} \times \sin \theta$$





CAPACIDADE DE CARGA

Equação de Terzaghi para sapata corrida
em situação drenada

$$\sigma_u = c' \times N_c + q' \times N_q + \frac{1}{2} \times \gamma' \times B \times N_\gamma$$

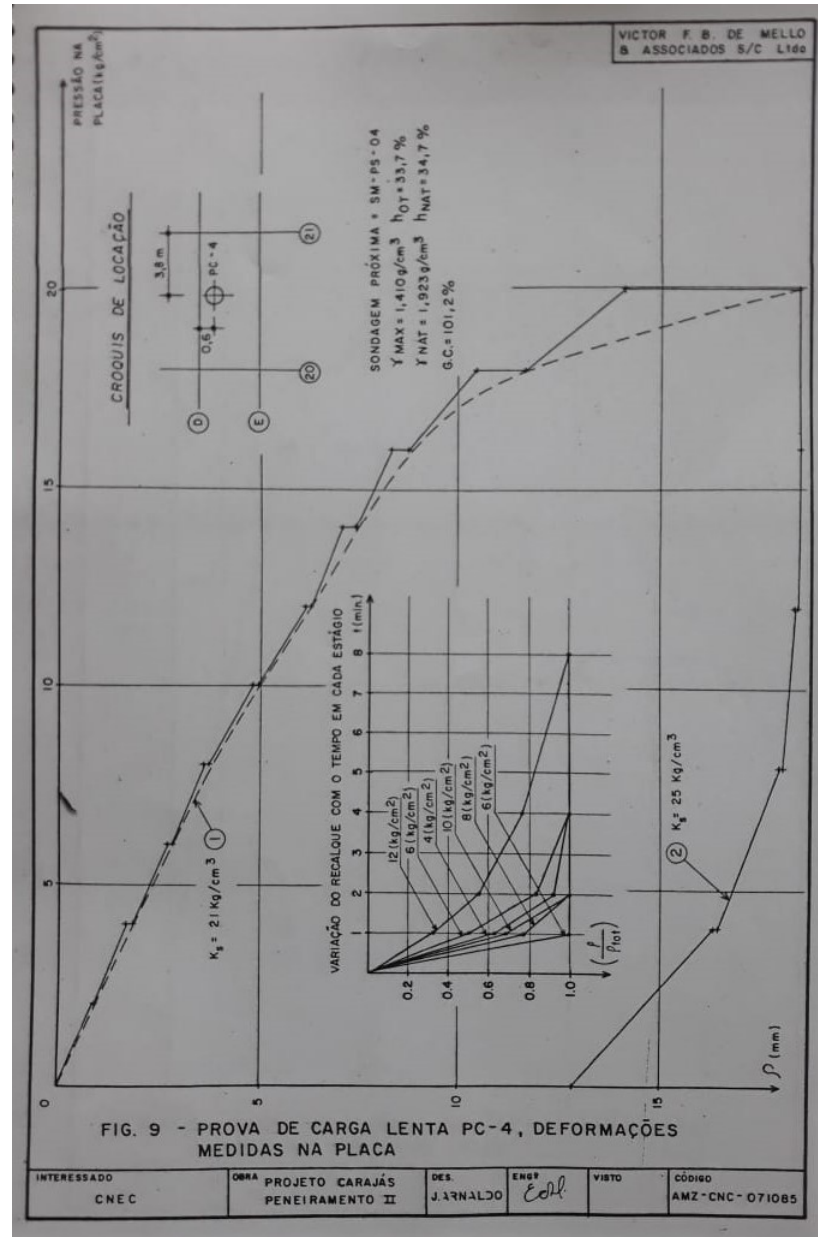
$$N_q = N_\varphi \times e^{\pi \times \tan \varphi}$$

$$N_c = (N_q - 1) \times \cot \varphi$$

$$N_\gamma = 2 \times (N_q - 1) \times \tan \varphi$$

$$N_\varphi = \frac{1 + \sin \varphi}{1 - \sin \varphi}$$

PROVA DE CARGA EM PLACA



Experimento numérico - Plaxis

Effective stresses
Extreme effective principal stress -345,58 kN/m²

Plate properties

Material set: Sapata
Material type: Elastic

Properties:

- EA : 1,200E+07 kN/m
- EI : 3,600E+05 kNm²/m
- d : 0,600 m
- w : 15,000 kN/m/m
- v : 0,200

General | Parameters | Interfaces

Material set: Solo
Material model: Mohr-Coulomb
Material type: Drained

General properties:

- γ_{unsat} : 18,000 kN/m³
- γ_{sat} : 20,000 kN/m³

General | Parameters | Interfaces

Stiffness:

- E_{ref} : 4,000E+04 kN/m²
- ν (nu) : 0,350

Strength:

- c_{ref} : 30,000 kN/m²
- ϕ (phi) : 28,000 °
- ψ (psi) : 0,000 °

Mohr-Coulomb - Solo

General | Parameters | Interfaces

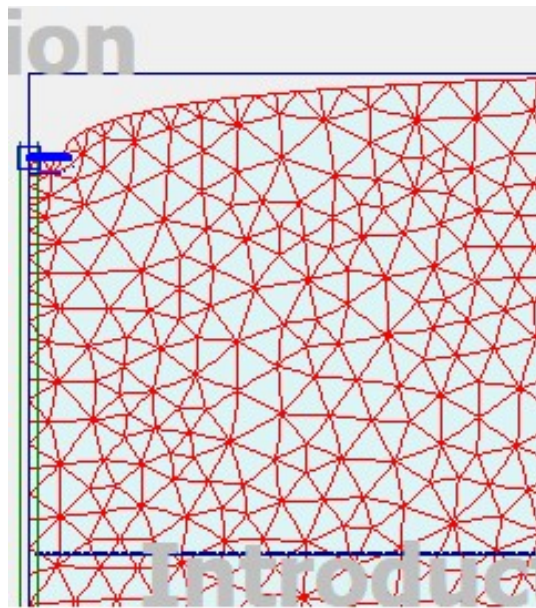
Strength:

- Rigid
- Manual

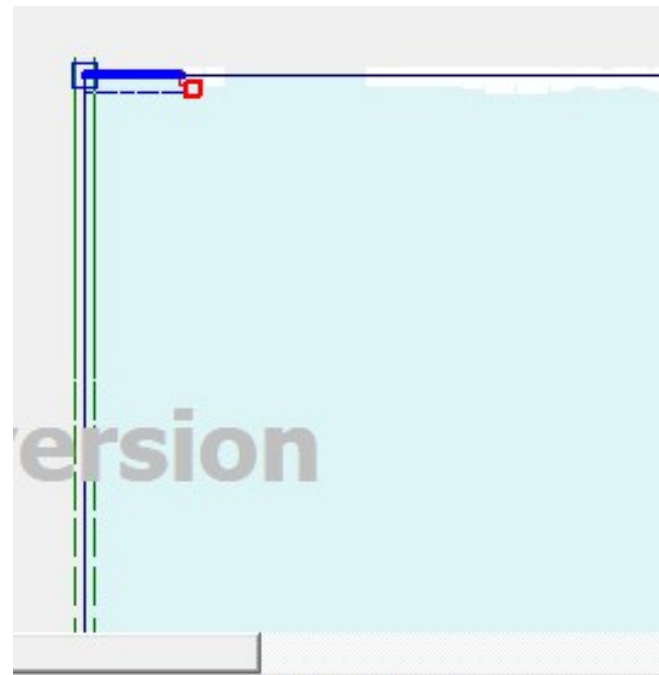
R_{inter} : 1,000

Real interface thickness:

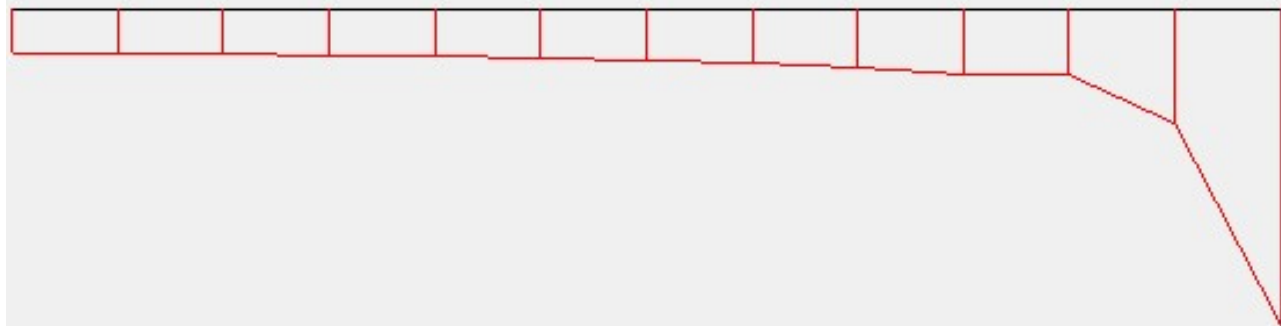
- δ_{inter} : 0,000



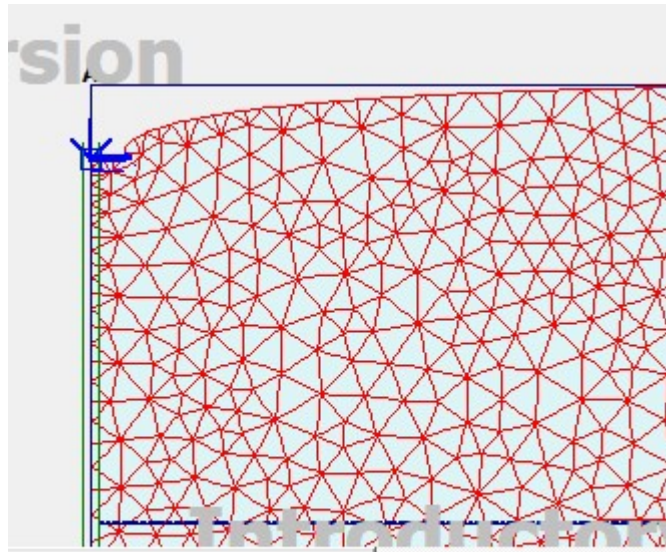
Deformed mesh
Extreme total displacement $10,57 \cdot 10^{-3}$ m
(displacements scaled up 200,00 times)



Plastic points
□ Mohr-Coulomb point □ Tension cut-off point

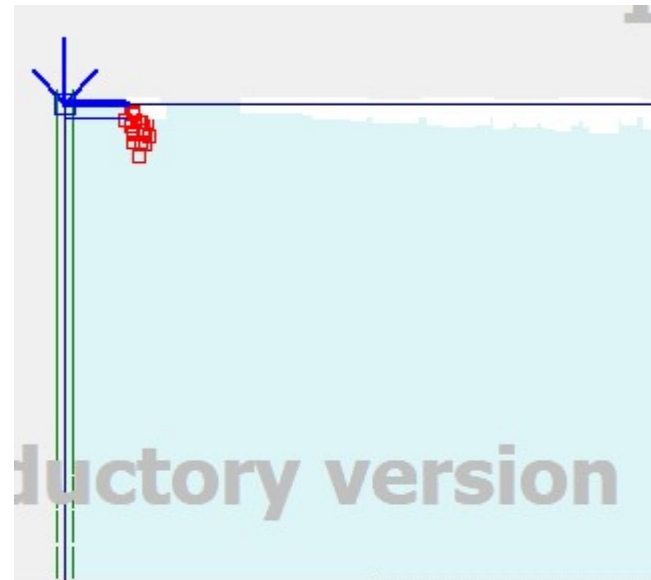


Effective normal stresses
Extreme effective normal stress $530,91 \text{ kN/m}^2$



Deformed mesh

Extreme total displacement $19,92 \cdot 10^{-3}$ m
(displacements scaled up 100,00 times)



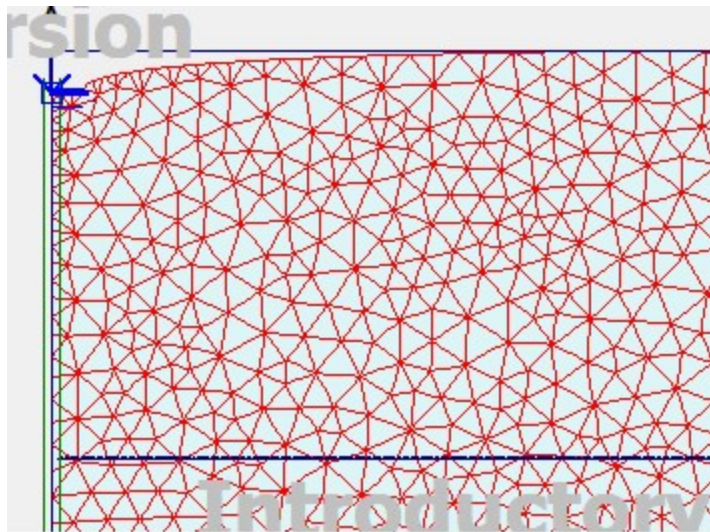
Plastic points

Mohr-Coulomb point
 Tension cut-off point



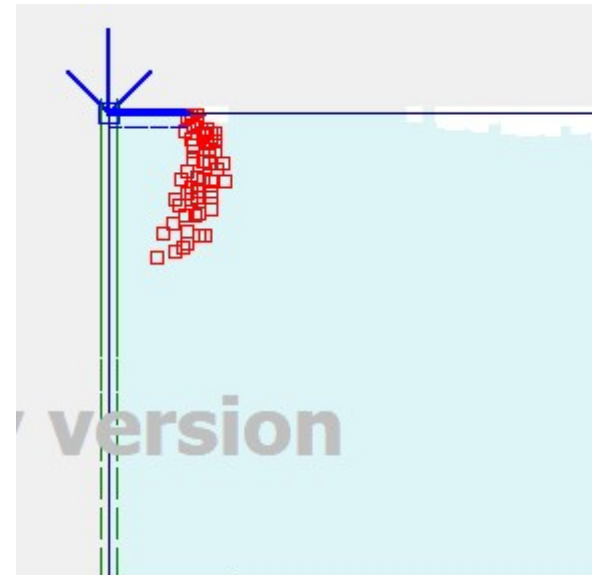
Effective normal stresses

Extreme effective normal stress $903,80 \text{ kN/m}^2$



Deformed mesh

Extreme total displacement $24,87 \cdot 10^{-3}$ m
 (displacements scaled up 50,00 times)



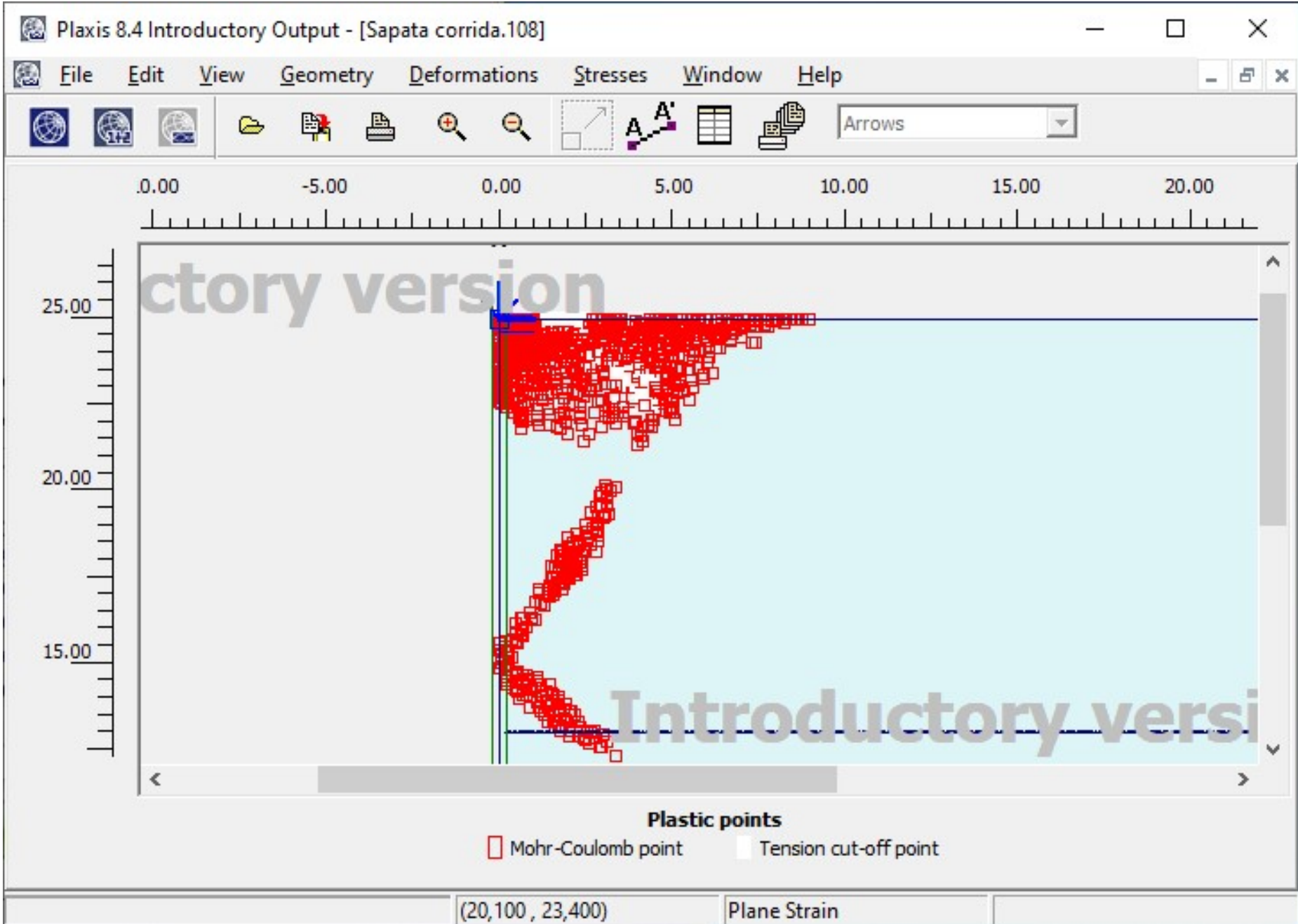
Plastic points

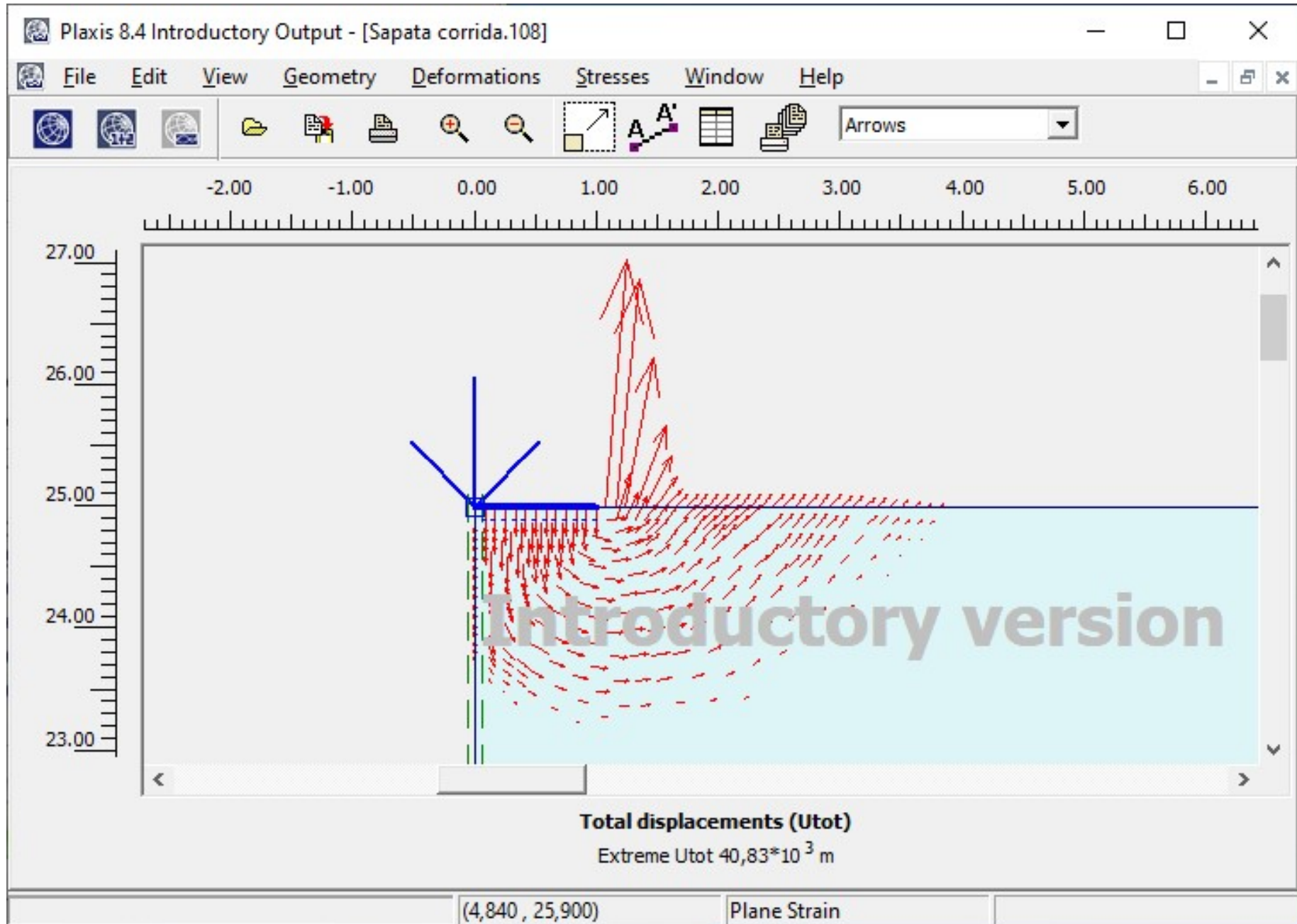
□ Mohr-Coulomb point
 Tension cut-off point



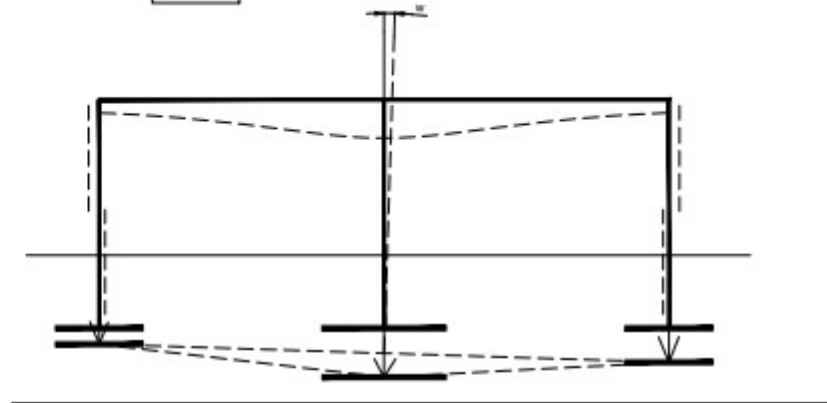
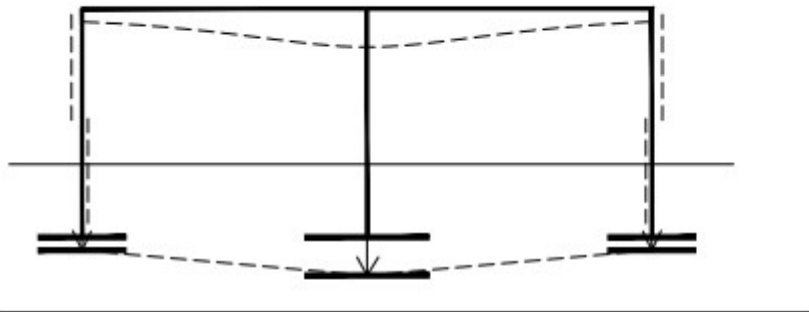
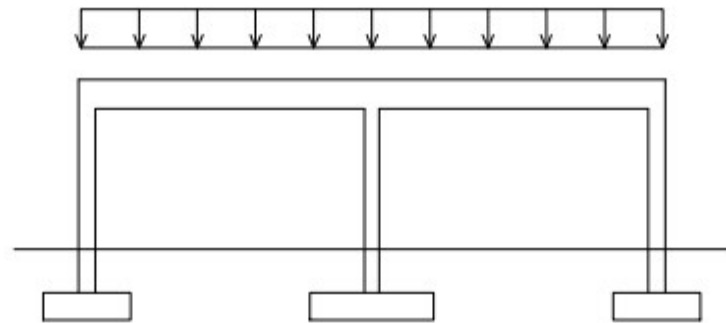
Effective normal stresses

Extreme effective normal stress $1,05 \cdot 10^3$ kN/m²

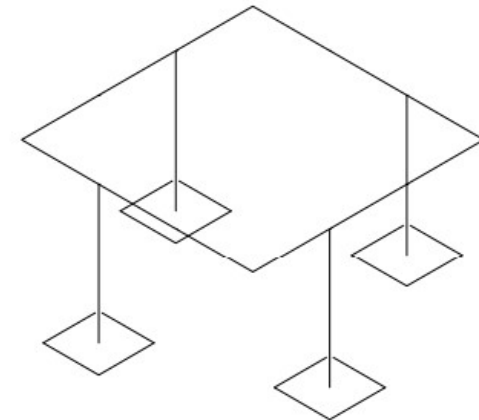
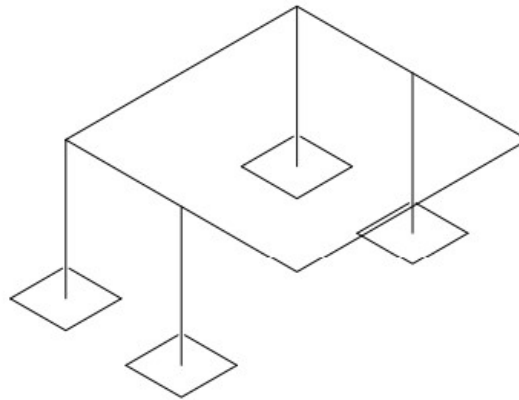
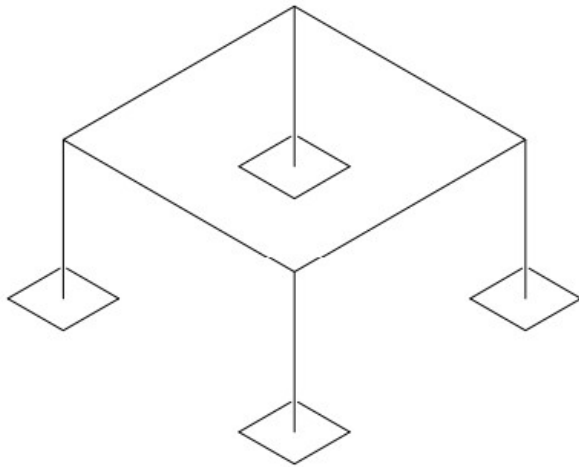
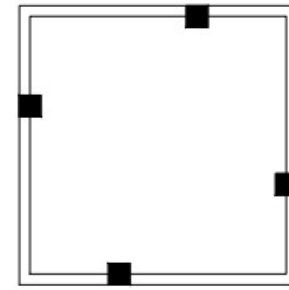
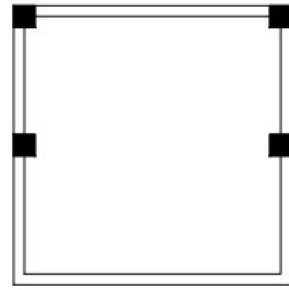
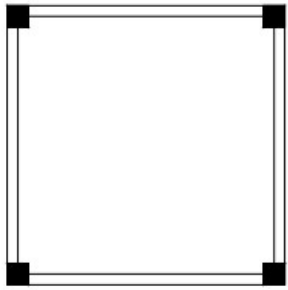




INTERRELAÇÃO ENTRE SOLO, FUNDAÇÃO E ESTRUTURA



Regularidade da estrutura



ELU / ELS

- Estados limites são estados a partir dos quais a estrutura apresenta desempenho inadequado às finalidades da construção;
- E. L. Último: associados com perda de equilíbrio, ruptura, instabilidade;
 - Determinam paralisação do uso;
- E. L. Serviço: associados com danos ligeiros, deformações, vibrações;
 - Impedem o uso normal ou comprometem a durabilidade;

CARGAS USUAIS NAS FUNDAÇÕES

- Ações a considerar no projeto:
 - Permanentes;
 - Variáveis;
 - Excepcionais;
- Alguns exemplos:
 - Edifícios: residenciais, comerciais, etc.;
 - Galpões para diversas finalidades;
 - Obras de arte especiais: galerias, pontes e viadutos;
 - Reservatórios;
 - Torres para diversos fins;
 - Outros;