Fluid Mechanics

Chapter 6:

Viscous Flow in Ducts

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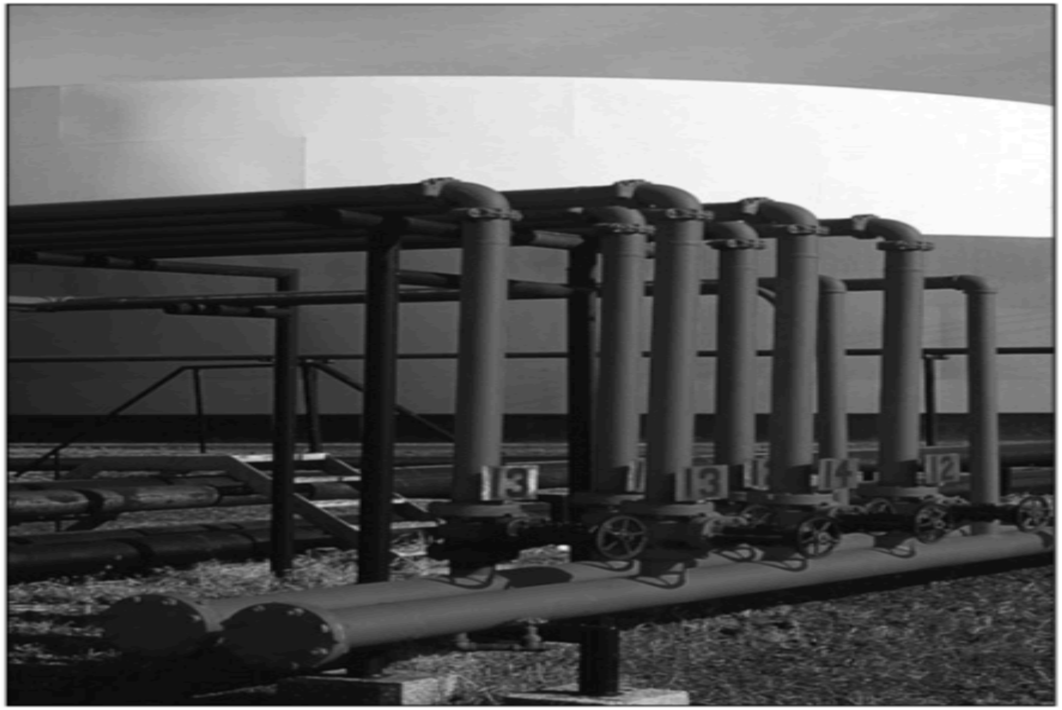


Steam pipe bridge in a geothermal power plant. Pipe flows are everywhere, often occurring in groups or networks. They are designed using the principles outlined in this chapter. *(Courtesy of Dr. E. R. Degginger/Color-Pic Inc.)*

Introduction

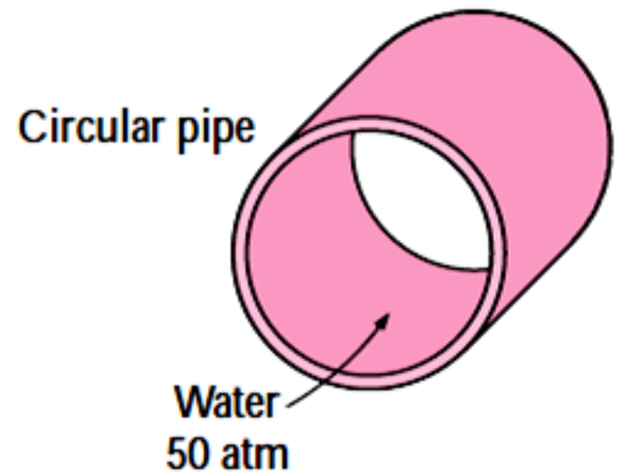
- Fluid flow in circular and noncircular pipes is commonly encountered in practice.
- The hot and cold water that we use in our homes is pumped through pipes. Water in a city is distributed by extensive piping networks. Oil and natural gas are transported hundreds of miles by large pipelines. Blood is carried throughout our bodies by arteries and veins. The cooling water in an engine is transported by hoses to the pipes in the radiator where it is cooled as it flows.
- The fluid in such applications is usually forced to flow by a fan or pump through a flow section.
- We pay particular attention to friction, which is directly related to the pressure drop and head loss during flow through pipes and ducts.

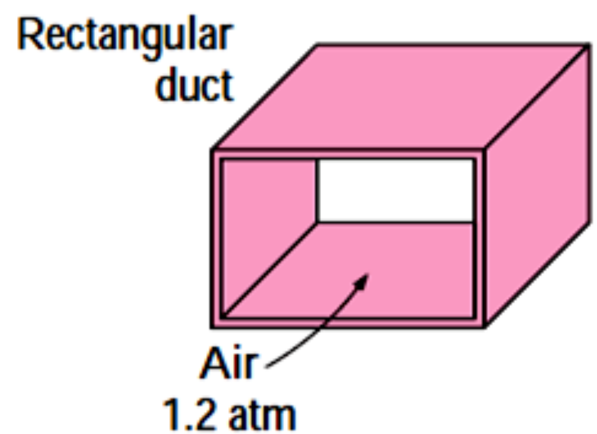
Introduction

- The pressure drop is then used to determine the pumping power requirement.
- A typical piping system involves pipes of different diameters connected to each other by various fittings or elbows to route the fluid, valves to control the flow rate, and pumps to pressurize the fluid.
- The terms ***pipe***, ***duct***, *and* ***conduit*** are usually used interchangeably for flow sections.
- In general, flow sections of circular cross section are referred to as pipes (especially when the fluid is a liquid), and flow sections of noncircular cross section as ducts (especially when the fluid is a gas) Small diameter pipes are usually referred to as tubes.

Introduction

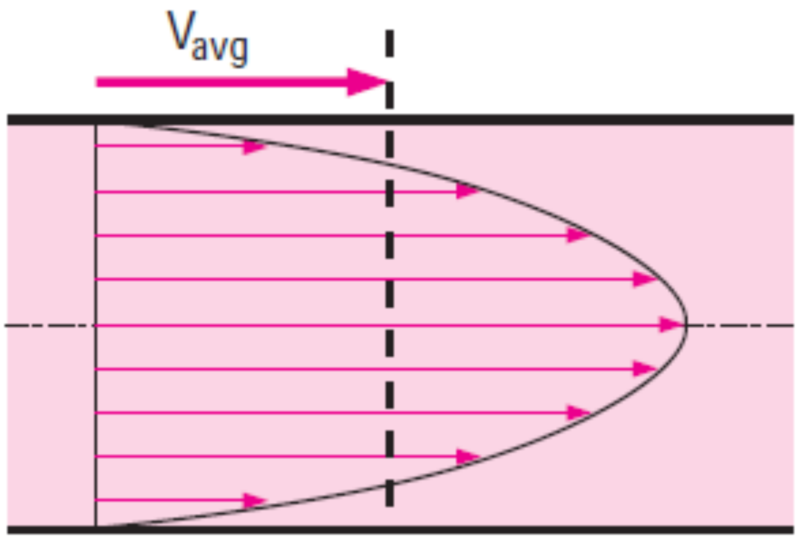
- Most fluids, especially liquids, are transported in circular pipes. This is because pipes with a circular cross section can withstand large pressure differences between the inside and the outside without undergoing significant distortion.
- Noncircular pipes are usually used in applications such as the heating and cooling systems of buildings where the pressure difference is relatively small, the manufacturing and installation costs are lower, and the available space is limited for ductwork.





Introduction

- The fluid velocity in a pipe changes from zero at the surface because of the no-slip condition to a maximum at the pipe center.
- In fluid flow, it is convenient to work with an average velocity V_{avg}, which remains constant in incompressible flow when the cross-sectional area of the pipe is constant.
- The change in average velocity due to change in density and temperature and due to friction is usually small and is thus disregarded in calculations.



Average velocity V_{avg} is defined as the average speed through a cross section. For fully developed laminar pipe flow, V_{avg} is half of maximum velocity.

Introduction

The value of the average velocity V_{avg} at some streamwise cross-section is determined from the requirement that the conservation of mass principle be satisfied. That is,

$$\dot{m} = \rho V_{avg} A_c = \int_{A_c} \rho u(r)\, dA_c$$

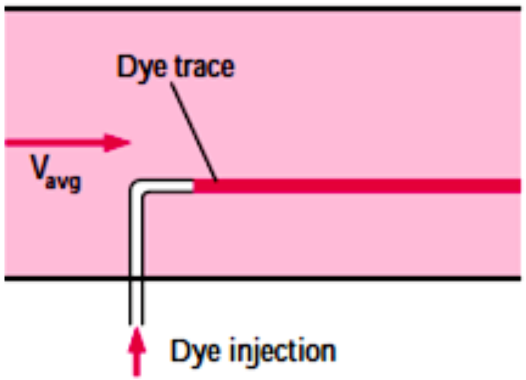
where $\dot{m}$ is the mass flow rate, ρ is the density, A_c is the cross-sectional area, and u(r) is the velocity profile. Then the average velocity for incompressible flow in a circular pipe of radius R can be expressed as

$$V_{avg} = \frac{\int_{A_c} \rho u(r)\, dA_c}{\rho A_c} = \frac{\int_0^R \rho u(r) 2\pi r\, dr}{\rho \pi R^2} = \frac{2}{R^2} \int_0^R u(r) r\, dr$$

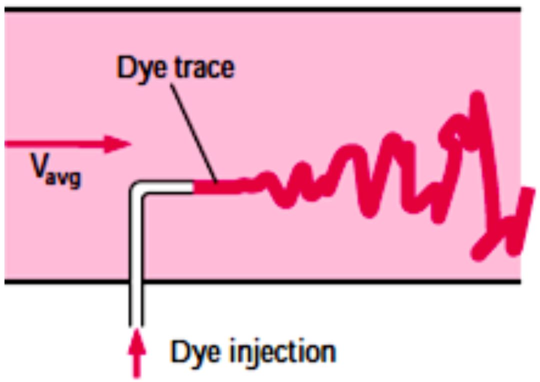
Therefore, when we know the flow rate or the velocity profile, the average velocity can be determined easily.

LAMINAR AND TURBULENT FLOWS

- Fluid flow in a pipe is streamlined at low velocities but turns chaotic as the velocity is increased above a critical value.
- A **laminar** flow is characterized **by smooth streamlines and highly ordered motion**, and **turbulent flow** is characterized by velocity fluctuations and highly disordered motion.
- The **transition from laminar to turbulent flow does not occur suddenly;** rather, it occurs over some region in which the flow fluctuates between laminar and turbulent flows before it becomes fully turbulent.
- Most flows encountered in practice are turbulent. Laminar flow is encountered when highly viscous fluids such as oils flow in small pipes or narrow passages.



(a) Laminar flow

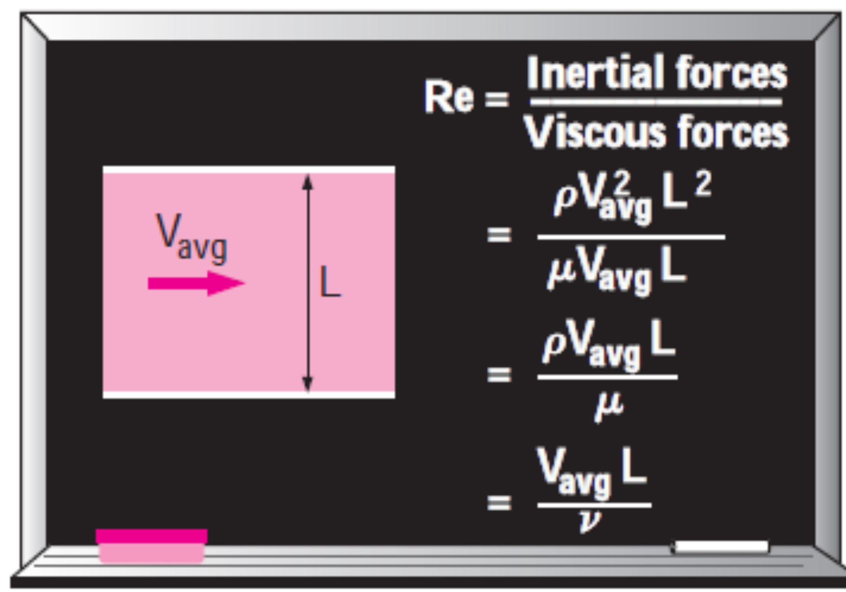


(b) Turbulent flow

Reynolds Number

- The transition from laminar to turbulent flow depends on the geometry, surface roughness, flow velocity, surface temperature, and type of fluid, among other things.
- After exhaustive experiments in the 1880s, Osborne Reynolds discovered that the flow regime depends mainly on the ratio of inertial forces to viscous forces in the fluid. This ratio is called the **Reynolds number** and is expressed for internal flow in a circular pipe as

$$Re = \frac{\text{Inertial forces}}{\text{Viscous forces}} = \frac{V_{avg}D}{\nu} = \frac{\rho V_{avg}D}{\mu}$$



Reynolds Number

- where V_{avg} = average flow velocity (m/s), D = characteristic length of the geometry (diameter in this case, in m), and $\nu = \mu/\rho$ = kinematic viscosity of the fluid (m^2/s).
- Note that the Reynolds number is a *dimensionless quantity.*
- At large Reynolds numbers, the inertial forces, which are proportional to the fluid density and the square of the fluid velocity, are large relative to the viscous forces, and thus the viscous forces cannot prevent the random and rapid fluctuations of the fluid.
- At small or moderate Reynolds numbers, however, the viscous forces are large enough to suppress these fluctuations and to keep the fluid "in line."
- Thus the flow is turbulent in the first case and laminar in the second.

Reynolds Number

- The Reynolds number at which the flow becomes turbulent is called the **critical Reynolds number, Re_{cr} .**
- The value of the critical Reynolds number is different for different geometries and flow conditions. For internal flow in a circular pipe, the generally accepted value of the critical Reynolds number is $Re_{cr} = 2300$.
- For flow through noncircular pipes, the Reynolds number is based on the **hydraulic diameter D_h** defined as

$$D_h = \frac{4A_c}{p}$$

- where A_c is the cross-sectional area of the pipe and p is its wetted perimeter. The hydraulic diameter is defined such that it reduces to ordinary diameter D for circular pipes,

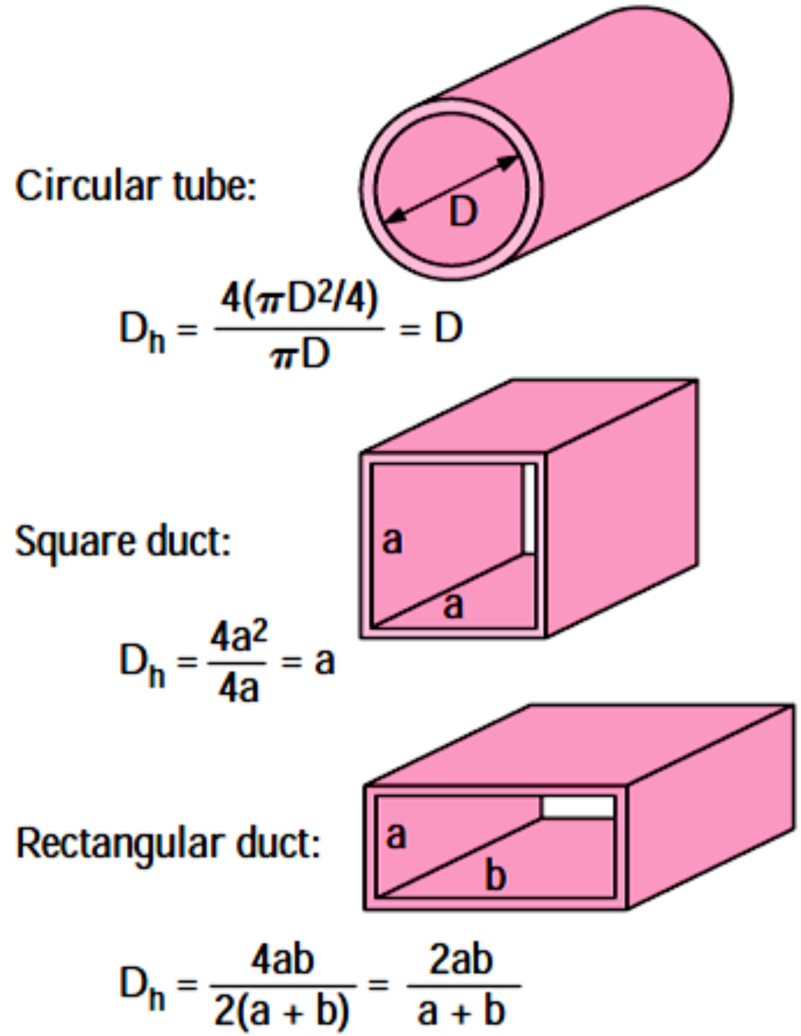
Reynolds Number

- For circular pipes

$$D_h = \frac{4A_c}{p} = \frac{4(\pi D^2/4)}{\pi D} = D$$

- For flow in circular pipes

$Re \lesssim 2300$	laminar flow
$2300 \lesssim Re \lesssim 4000$	transitional flow
$Re \gtrsim 4000$	turbulent flow

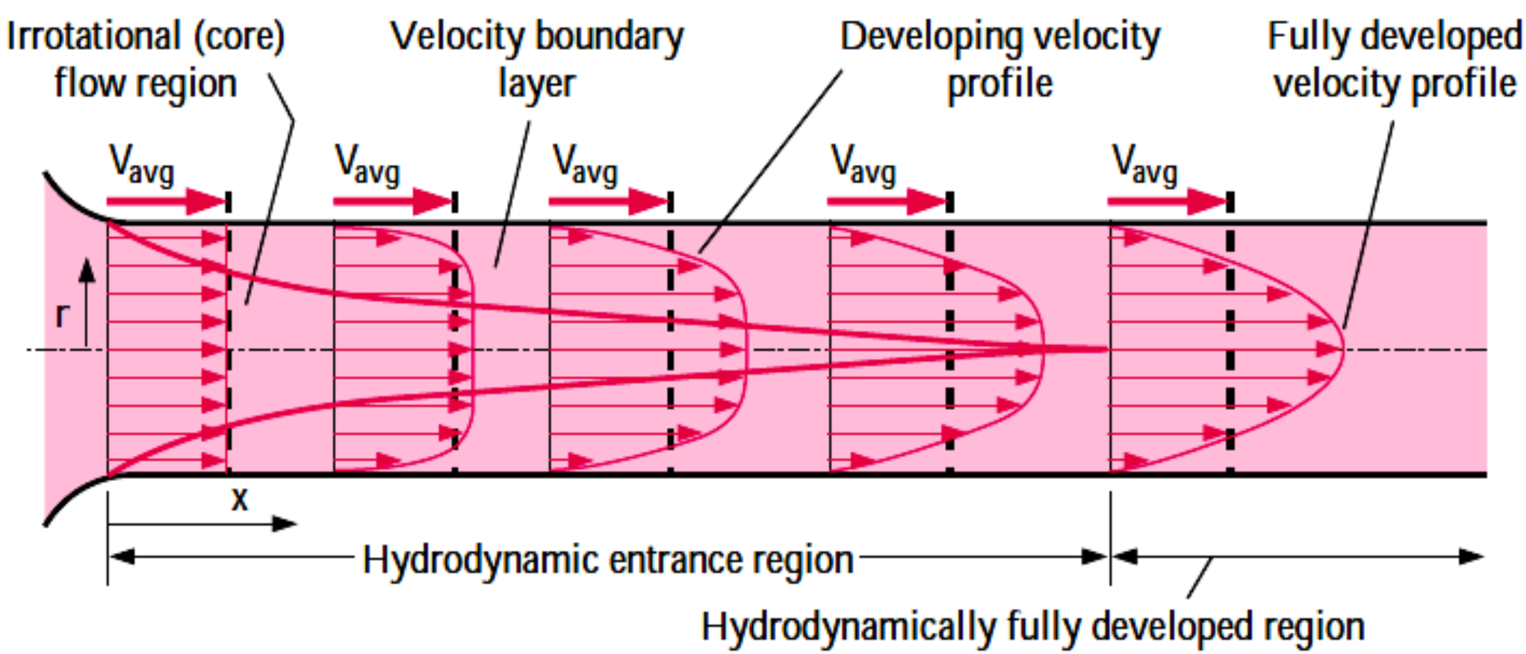


The Entrance Region

- Consider a fluid entering a circular pipe at a uniform velocity. Because of the no-slip condition, the fluid particles in the layer in contact with the surface of the pipe come to a complete stop.
- This layer also causes the fluid particles in the adjacent layers to slow down gradually as a result of friction.
- The region of the flow in which the effects of the viscous shearing forces caused by fluid viscosity are felt is called the **velocity boundary layer or** just the **boundary layer.**
- **The hypothetical boundary surface divides the** flow in a pipe into two regions: the **boundary layer region,** in which the viscous effects and the velocity changes are significant, and the **irrotational (core) flow region,** in which the **frictional effects are negligible** and the velocity remains essentially constant in the radial direction.

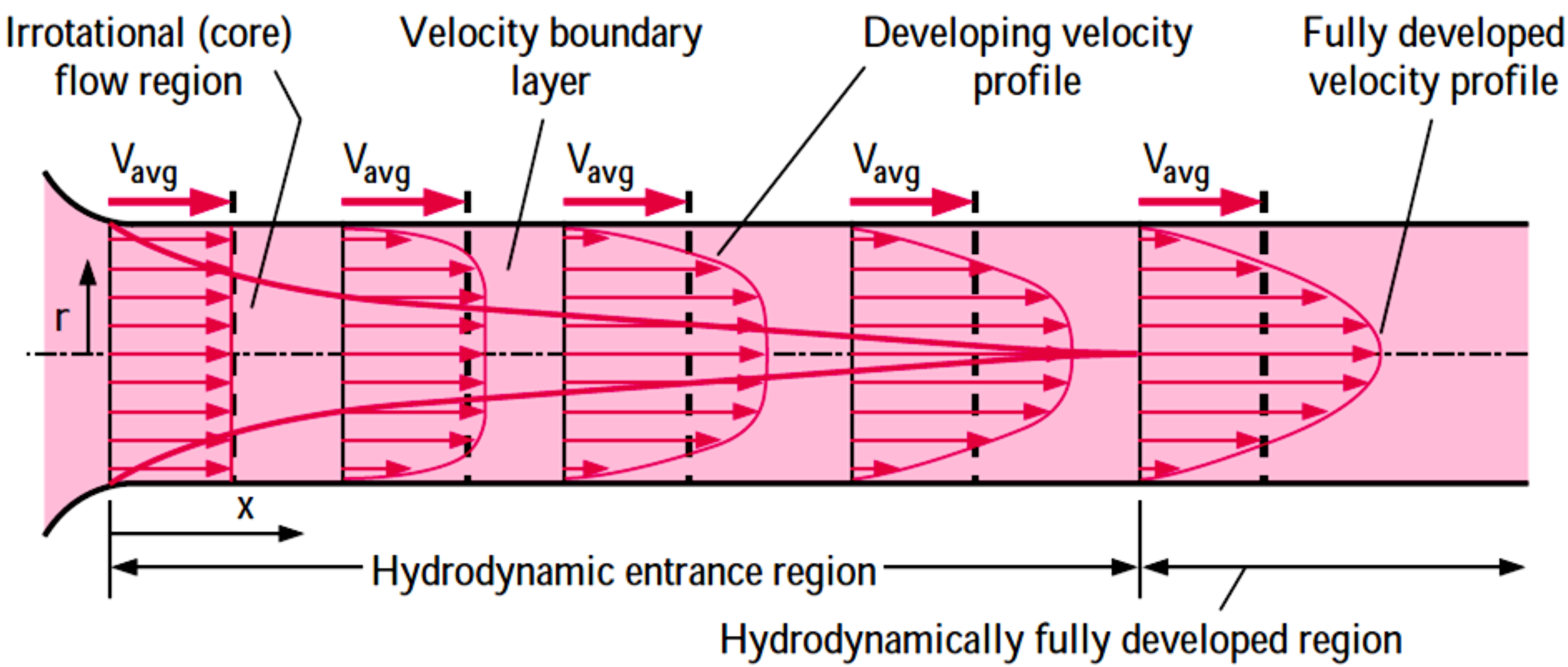
The Entrance Region

- The thickness of this boundary layer increases in the flow direction until the boundary layer reaches the pipe center and thus fills the entire pipe.
- The region from the pipe inlet to the point at which the boundary layer merges at the centerline is called the **hydrodynamic entrance region,** and the length of this region is called the **hydrodynamic entry length L_h.**



The Entrance Region

- Flow in the entrance region is called **hydrodynamically developing** flow since this is the region where the velocity profile develops.
- The region beyond the entrance region in which the velocity profile is fully developed and remains unchanged is called the **hydrodynamically fully developed** region.



The Entrance Region

- The velocity profile in the fully developed region is parabolic in laminar flow and somewhat flatter (or fuller) in turbulent flow due to eddy motion and more vigorous mixing in the radial direction.

Entry Lengths

- The hydrodynamic entry length is usually taken to be the distance from the pipe entrance to where the wall shear stress (and thus the friction factor) reaches within about 2 percent of the fully developed value.
- In *laminar flow,* the hydrodynamic entry length is given approximately as

$$L_{h,\ laminar} \cong 0.05 ReD$$

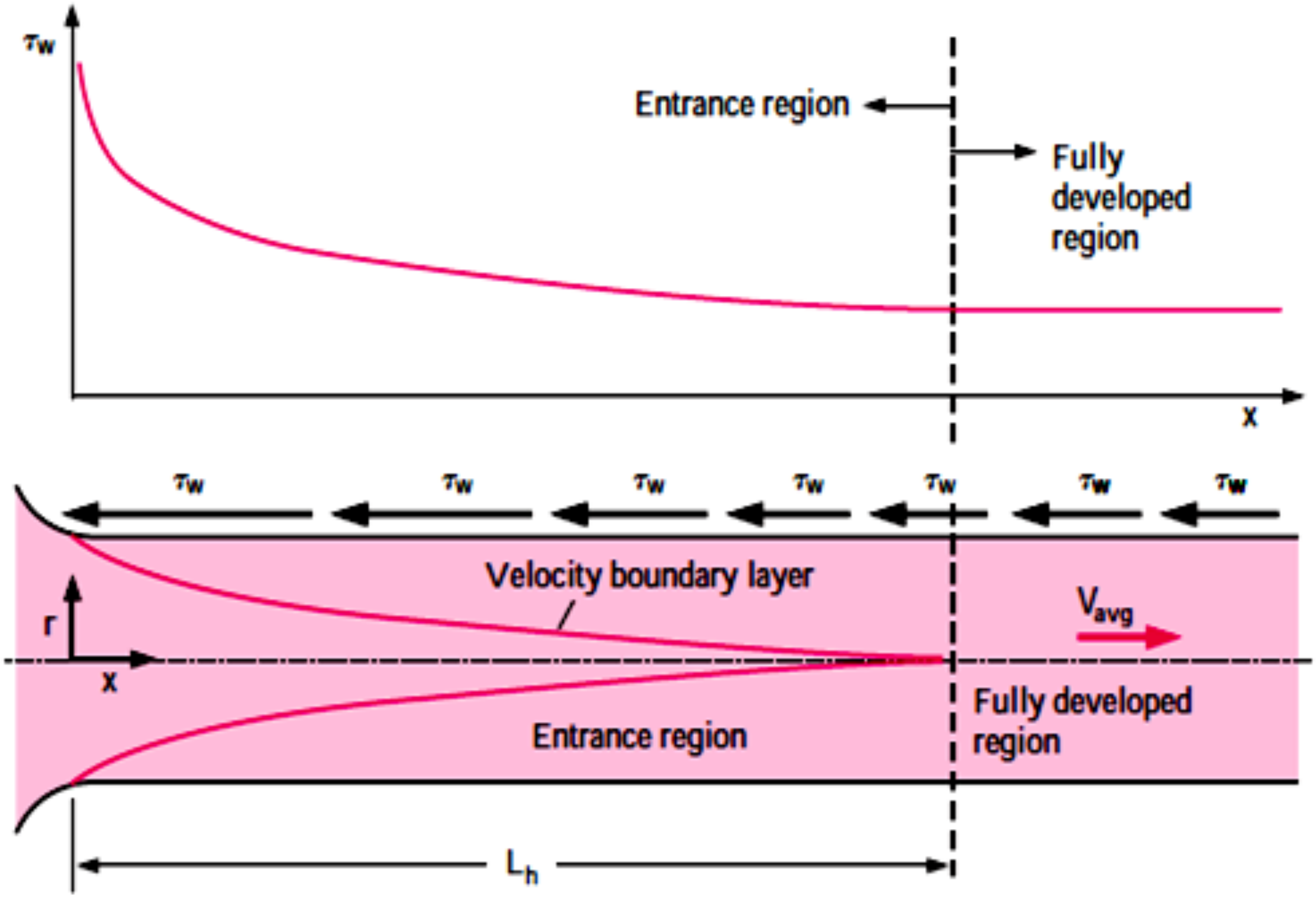


Fig. The variation of wall shear stress in the flow direction for flow in a pipe from the entrance region into the fully developed region.

The Entrance Region

- In turbulent flow, the intense mixing during random fluctuations usually overshadows the effects of molecular diffusion.
- The hydrodynamic entry length for turbulent flow can be approximated as [see Bhatti and Shah (1987) and Zhi-qing (1982)]

$$L_{h,\ turbulent} = 1.359 D Re_D^{1/4}$$

- The entry length is much shorter in turbulent flow, as expected, and its dependence on the Reynolds number is weaker.
- In many pipe flows of practical engineering interest, the entrance effects become insignificant beyond a pipe length of 10 diameters, and the hydrodynamic entry length is approximated as

The Entrance Region

$$L_{h,\ turbulent} \approx 10D$$

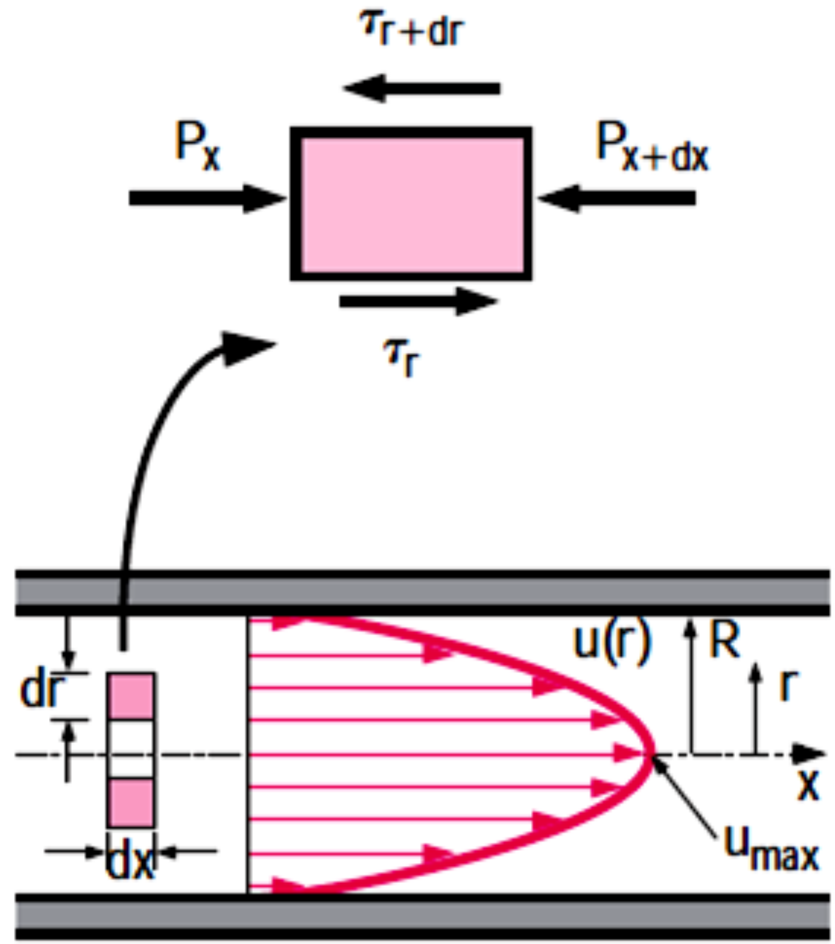
- The pipes used in practice are usually several times the length of the entrance region, and thus the flow through the pipes is often assumed to be fully developed for the entire length of the pipe. This simplistic approach gives reasonable results for long pipes but sometimes poor results for short ones since it under predicts the wall shear stress and thus the friction factor.

Laminar Flow in Pipes

- Flow in pipes is laminar for Re $\leq$ 2300, and that the flow is fully developed if the pipe is sufficiently long (relative to the entry length) so that the entrance effects are negligible.
- In this section we consider the steady laminar flow of an incompressible fluid with constant properties in the fully developed region of a straight circular pipe.
- In fully developed laminar flow, each fluid particle moves at a constant axial velocity along a streamline and the velocity profile u(r) remains unchanged in the flow direction. There is no motion in the radial direction, and thus the velocity component in the direction normal to flow is everywhere zero. There is no acceleration since the flow is steady and fully developed.

Laminar Flow in Pipes

- Consider a ring-shaped differential volume element of radius r, thickness dr, and length dx oriented coaxially with the pipe, as shown in the Fig.
- The volume element involves only pressure and viscous effects and thus the pressure and shear forces must balance each other.
- The pressure force acting on a submerged plane surface is the product of the pressure at the centroid of the surface and the surface area.



Laminar Flow in Pipes

- A force balance on the volume element in the flow direction gives

$$(2\pi r\, dr\, P)_x - (2\pi r\, dr\, P)_{x+dx} + (2\pi r\, dx\, \tau)_r - (2\pi r\, dx\, \tau)_{r+dr} = 0$$

- which indicates that in fully developed flow in a horizontal pipe, the viscous and pressure forces balance each other. Dividing by $2\pi drdx$ *and* rearranging,

$$r\frac{P_{x+dx} - P_x}{dx} + \frac{(r\tau)_{r+dr} - (r\tau)_r}{dr} = 0$$

Taking the limit as dr, dx $\rightarrow$ 0 gives

$$r\frac{dP}{dx} + \frac{d(r\tau)}{dr} = 0$$

Laminar Flow in Pipes

Substituting $\tau = -\mu(du/dr)$ and taking μ = constant gives the desired equation,

$$\frac{\mu}{r}\frac{d}{dr}\left(r\frac{du}{dr}\right) = \frac{dP}{dx}$$

- The quantity du/dr is negative in pipe flow, and the negative sign is included to obtain positive values for τ.
  (Or, du/dr = -du/dy since y = R - r)
- Rearranging and integrating twice gives

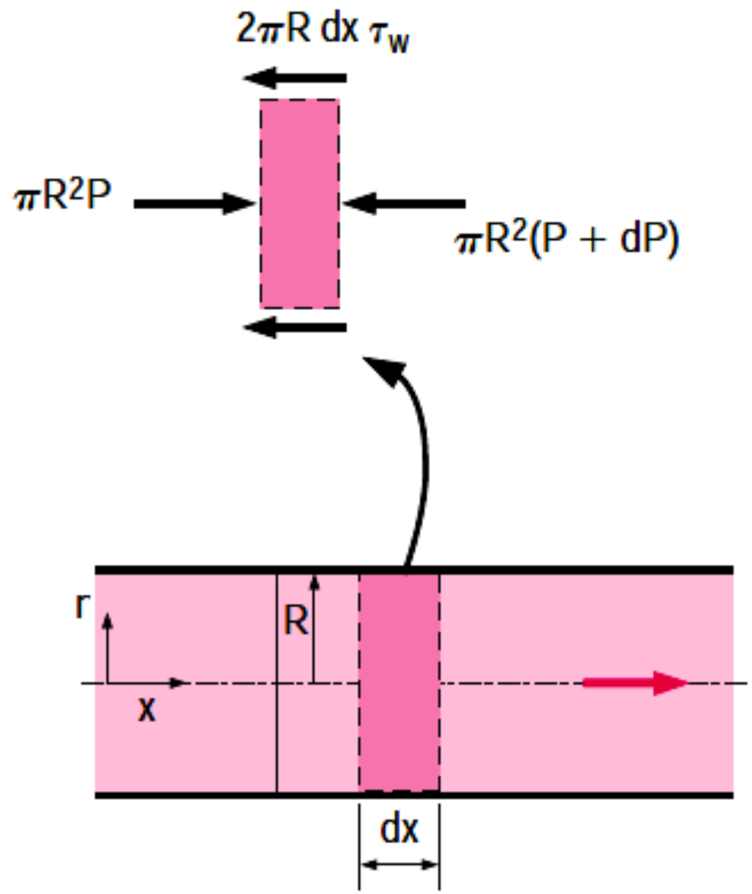
$$u(r) = \frac{1}{4\mu}\left(\frac{dP}{dx}\right) + C_1 \ln r + C_2$$

Laminar Flow in Pipes

- Writing a force balance on a volume element of radius R and thickness dx (a slice of the pipe), gives

$$\frac{dP}{dx} = -\frac{2\tau_w}{R}$$

- Here τ_w is constant since the viscosity and the velocity profile are constants in the fully developed region.
- Therefore, dP/dx = constant.



Force balance:

$$\pi R^2 P - \pi R^2 (P + dP) - 2\pi R \, dx \, \tau_w = 0$$

Simplifying:

$$\frac{dP}{dx} = -\frac{2\tau_w}{R}$$

Laminar Flow in Pipes

- The velocity profile u(r) is obtained by applying the boundary conditions $\partial u/\partial r = 0$ at r = 0 (because of symmetry about the centerline) and u = 0 at r = R (the no-slip condition at the pipe surface). We get

$$u(r) = -\frac{R^2}{4\mu}\left(\frac{dP}{dx}\right)\left(1 - \frac{r^2}{R^2}\right)$$

- Therefore, the velocity profile in fully developed laminar flow in a pipe is parabolic with a maximum at the centerline and minimum (zero) at the pipe wall.
- Also, the axial velocity u is positive for any r, and thus the axial pressure gradient dP/dx must be negative (i.e., pressure must decrease in the flow direction because of viscous effects).

Laminar Flow in Pipes

- The average velocity is determined from its definition by substituting u(r) and performing the integration. It gives

$$V_{avg} = \frac{2}{R^2}\int_0^R u(r)r\,dr = \frac{-2}{R^2}\int_0^R \frac{R^2}{4\mu}\left(\frac{dP}{dx}\right)\left(1-\frac{r^2}{R^2}\right)r\,dr = -\frac{R^2}{8\mu}\left(\frac{dP}{dx}\right)$$

- Combining the last two equations, the velocity profile is rewritten as

$$u(r) = 2V_{avg}\left(1-\frac{r^2}{R^2}\right)$$

- This is a convenient form for the velocity profile since V_{avg} can be determined easily from the flow rate information.
- The maximum velocity occurs at the centerline and is determined by substituting $r = 0$,

$$u_{max} = 2V_{avg}$$

Pressure Drop and Head Loss

- A quantity of interest in the analysis of pipe flow is the pressure drop ΔP since it is directly related to the power requirements of the fan or pump to maintain flow.
- We note that dP/dx = constant, and integrating from $x = x_1$ where the pressure is P_1 to $x = x_1 + L$ where the pressure is P_2 gives

$$\frac{dP}{dx} = \frac{P_2 - P_1}{L}$$

- Substituting this into the V_{avg} expression, the pressure drop can be expressed as

Laminar flow: $$\Delta P = P_1 - P_2 = \frac{8\mu L V_{avg}}{R^2} = \frac{32\mu L V_{avg}}{D^2}$$

Pressure Drop and Head Loss

- Pressure drop due to viscous effects represents an irreversible pressure loss, and it is called **pressure loss** ΔP_L to emphasize that it is a loss.
- Pressure drop is proportional to the viscosity μ of the fluid, and ΔP would be zero if there were no friction. Therefore, the drop of pressure from P_1 to P_2 in this case is due entirely to viscous effects.
- In practice, it is found convenient to express the pressure loss for all types of fully developed internal flows (laminar or turbulent flows, circular or noncircular pipes, smooth or rough surfaces, horizontal or inclined pipes) as

Pressure loss: $$\Delta P_L = f \frac{L}{D} \frac{\rho V_{avg}^2}{2}$$

Pressure Drop and Head Loss

- where $\rho V^2_{avg}/2$ is the dynamic pressure and f is the **Darcy friction factor** also called the **Darcy–Weisbach friction factor**

$$f = \frac{8\tau_w}{\rho V_{avg}^2}$$

- For fully developed laminar flow in a circular pipe solving for f gives

Circular pipe, laminar: $$f = \frac{64\mu}{\rho D V_{avg}} = \frac{64}{Re}$$

- This equation shows that in laminar flow, the friction factor is a function of the Reynolds number only and is independent of the roughness of the pipe surface.

Pressure Drop and Head Loss

- In the analysis of piping systems, pressure losses are commonly expressed in terms of the equivalent fluid column height, called the **head loss h_L.**
- Noting from fluid statics that $\Delta P = \rho g h$ and thus a pressure difference of ΔP corresponds to a fluid height of $h = \Delta P/\rho g$, the pipe head loss is obtained by dividing ΔP_L by ρg to give

Head loss: $$h_L = \frac{\Delta P_L}{\rho g} = f \frac{L}{D} \frac{V_{avg}^2}{2g}$$

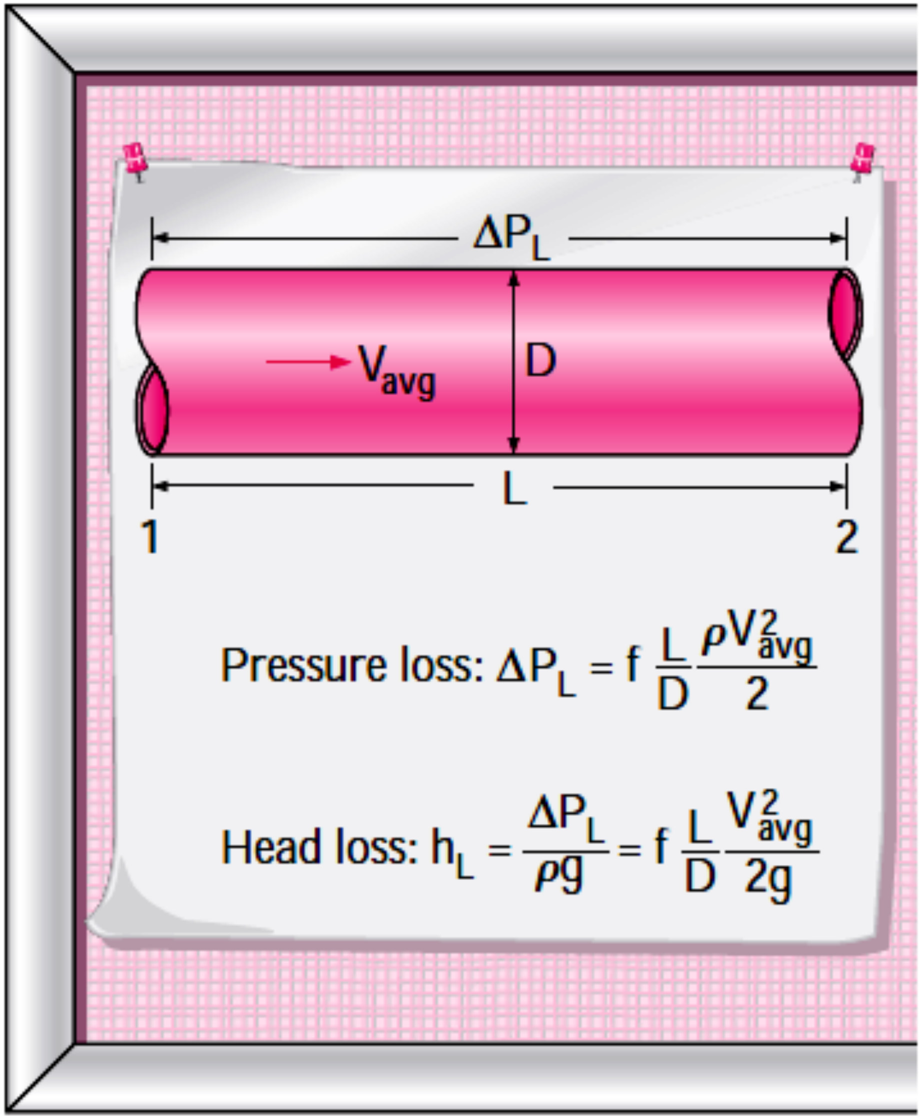
- The head loss h_L represents the additional height that the fluid needs to be raised by a pump in order to overcome the frictional losses in the pipe. The head loss is caused by viscosity, and it is directly related to the wall shear stress.

Pressure Drop and Head Loss

- Once the pressure loss (or head loss) is known, the required pumping power to overcome the pressure loss is determined from

$$\dot{W}_{\text{pump, L}} = \dot{V}\,\Delta P_L = \dot{V}\rho g h_L = \dot{m} g h_L$$

where $\dot{V}$ is the volume flow rate and $\dot{m}$ is the mass flow rate.



Pressure Drop and Head Loss

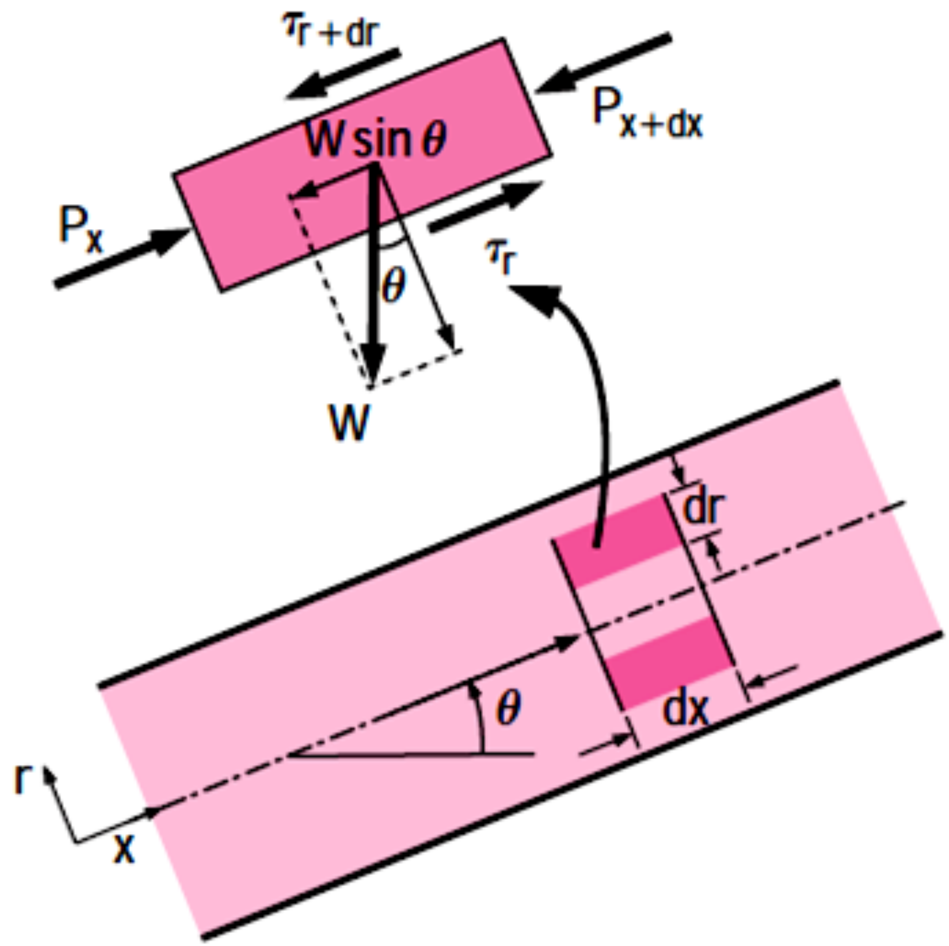
- The average velocity for laminar flow in a horizontal pipe is

Horizontal pipe: $$V_{avg} = \frac{(P_1 - P_2)R^2}{8\mu L} = \frac{(P_1 - P_2)D^2}{32\mu L} = \frac{\Delta P\, D^2}{32\mu L}$$

- Then the volume flow rate for laminar flow through a horizontal pipe of diameter *D and length L becomes*

$$\dot{V} = V_{avg} A_c = \frac{(P_1 - P_2)R^2}{8\mu L}\pi R^2 = \frac{(P_1 - P_2)\pi D^4}{128\mu L} = \frac{\Delta P\, \pi D^4}{128\mu L}$$

Inclined Pipes



- Relations for inclined pipes can be obtained in a similar manner from a force balance in the direction of flow. The only additional force in this case is the component of the fluid weight in the flow direction, whose magnitude is

$$W_x = W \sin\theta = \rho g V_{element} \sin\theta = \rho g (2\pi r\, dr\, dx) \sin\theta$$

where θ is the angle between the horizontal and the flow direction

Inclined Pipes

- The force balance now becomes

$$(2\pi r\,dr\,P)_x - (2\pi r\,dr\,P)_{x+dx} + (2\pi r\,dx\,\tau)_r - (2\pi r\,dx\,\tau)_{r+dr} - \rho g(2\pi r\,dr\,dx)\sin\theta = 0$$

which results in the differential equation

$$\frac{\mu}{r}\frac{d}{dr}\left(r\frac{du}{dr}\right) = \frac{dP}{dx} + \rho g\sin\theta$$

- Following the same solution procedure, the velocity profile can be shown to be

$$u(r) = -\frac{R^2}{4\mu}\left(\frac{dP}{dx} + \rho g\sin\theta\right)\left(1 - \frac{r^2}{R^2}\right)$$

Inclined Pipes

- It can also be shown that the average velocity and the volume flow rate relations for laminar flow through inclined pipes are, respectively,

$$V_{avg} = \frac{(\Delta P - \rho g L \sin\theta)D^2}{32\mu L} \quad \text{and} \quad \dot{V} = \frac{(\Delta P - \rho g L \sin\theta)\pi D^4}{128\mu L}$$

- which are identical to the corresponding relations for horizontal pipes, except that ΔP is replaced by $\Delta P - \rho g L \sin\theta$.
- Therefore, the results already obtained for horizontal pipes can also be used for inclined pipes provided that ΔP is replaced by $\Delta P - \rho g L \sin\theta$.
- Note that $\theta > 0$ and thus $\sin\theta > 0$ for uphill flow, and $\theta < 0$ and thus $\sin\theta < 0$ for downhill flow.

Inclined Pipes

- In inclined pipes, the combined effect of pressure difference and gravity drives the flow. Gravity helps downhill flow but opposes uphill flow. Therefore, much greater pressure differences need to be applied to maintain a specified flow rate in uphill flow although this becomes important only for liquids, because the density of gases is generally low.

Horizontal pipe: $\dot{V} = \dfrac{\Delta P \, \pi D^4}{128 \mu L}$

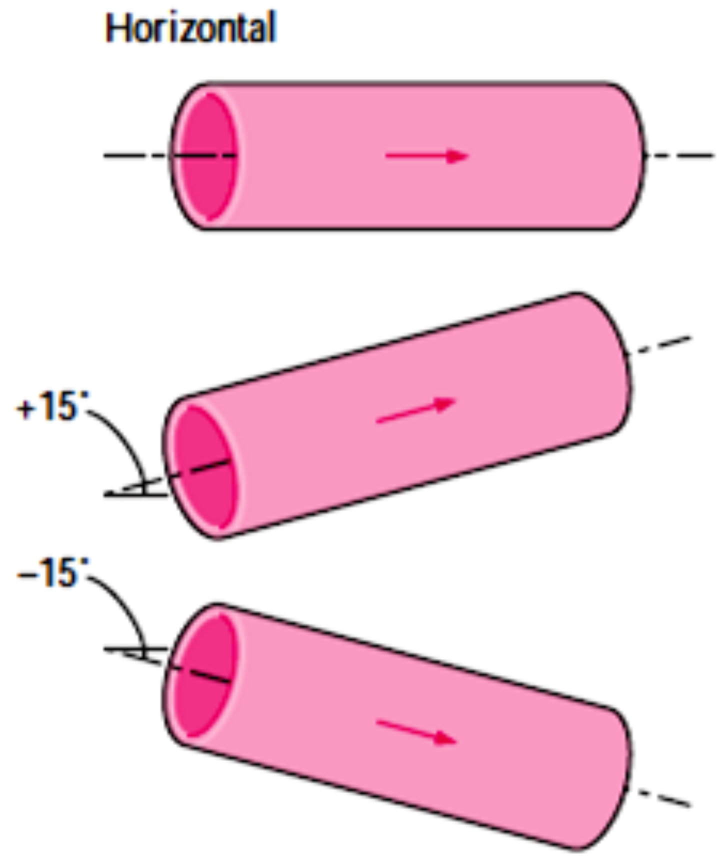
Inclined pipe: $\dot{V} = \dfrac{(\Delta P - \rho g L \sin\theta)\pi D^4}{128 \mu L}$

Uphill flow: $\theta > 0$ and $\sin\theta > 0$

Downhill flow: $\theta < 0$ and $\sin\theta < 0$

EXAMPLE 1.
Flow Rates in Horizontal and Inclined Pipes

- Oil at 20◦C (ρ = 888 kg/m^3 and μ = 0.800 kg/m · s) is flowing steadily through a 5-cm-diameter 40-m-long pipe . The pressure at the pipe inlet and outlet are measured to be 745 and 97 kPa, respectively. Determine the flow rate of oil through the pipe assuming the pipe is (a) horizontal, (b) inclined 15◦ upward, (c) inclined 15◦ downward. Also verify that the flow through the pipe is laminar.



SOLUTION The pressure readings at the inlet and outlet of a pipe are given. The flow rates are to be determined for three different orientations, and the flow is to be shown to be laminar.

Assumptions **1** The flow is steady and incompressible. **2** The entrance effects are negligible, and thus the flow is fully developed. **3** The pipe involves no components such as bends, valves, and connectors. **4** The piping section involves no work devices such as a pump or a turbine.

Properties The density and dynamic viscosity of oil are given to be $\rho = 888\ \text{kg/m}^3$ and $\mu = 0.800\ \text{kg/m}\cdot\text{s}$, respectively.

Analysis The pressure drop across the pipe and the pipe cross-sectional area are

$$\Delta P = P_1 - P_2 = 745 - 97 = 648\ \text{kPa}$$

$$A_c = \pi D^2/4 = \pi(0.05\ \text{m})^2/4 = 0.001963\ \text{m}^2$$

(a) The flow rate for all three cases can be determined from

$$\dot{V} = \frac{(\Delta P - \rho g L \sin\theta)\pi D^4}{128\mu L}$$

where θ is the angle the pipe makes with the horizontal. For the horizontal case, $\theta = 0$ and thus $\sin\theta = 0$. Therefore,

$$\dot{V}_{\text{horiz}} = \frac{\Delta P\, \pi D^4}{128\mu L} = \frac{(648\ \text{kPa})\pi(0.05\ \text{m})^4}{128(0.800\ \text{kg/m}\cdot\text{s})(40\ \text{m})}\left(\frac{1000\ \text{N/m}^2}{1\ \text{kPa}}\right)\left(\frac{1\ \text{kg}\cdot\text{m/s}^2}{1\ \text{N}}\right)$$

$$= \mathbf{0.00311\ m^3/s}$$

(*b*) For uphill flow with an inclination of 15°, we have $\theta = +15°$, and

$$\dot{V}_{\text{uphill}} = \frac{(\Delta P - \rho g L \sin\theta)\pi D^4}{128\mu L}$$

$$= \frac{[648{,}000\ \text{Pa} - (888\ \text{kg/m}^3)(9.81\ \text{m/s}^2)(40\ \text{m})\sin 15°]\pi(0.05\ \text{m})^4}{128(0.800\ \text{kg/m}\cdot\text{s})(40\ \text{m})}\left(\frac{1\ \text{kg}\cdot\text{m/s}^2}{1\ \text{Pa}\cdot\text{m}^2}\right)$$

$$= \mathbf{0.00267\ m^3/s}$$

(*c*) For downhill flow with an inclination of 15°, we have $\theta = -15°$, and

$$\dot{V}_{\text{downhill}} = \frac{(\Delta P - \rho g L \sin\theta)\pi D^4}{128\mu L}$$

$$= \frac{[648{,}000\ \text{Pa} - (888\ \text{kg/m}^3)(9.81\ \text{m/s}^2)(40\ \text{m})\sin(-15°)]\pi(0.05\ \text{m})^4}{128(0.800\ \text{kg/m}\cdot\text{s})(40\ \text{m})}\left(\frac{1\ \text{kg}\cdot\text{m/s}^2}{1\ \text{Pa}\cdot\text{m}^2}\right)$$

$$= \mathbf{0.00354\ m^3/s}$$

The flow rate is the highest for the downhill flow case, as expected. The average fluid velocity and the Reynolds number in this case are

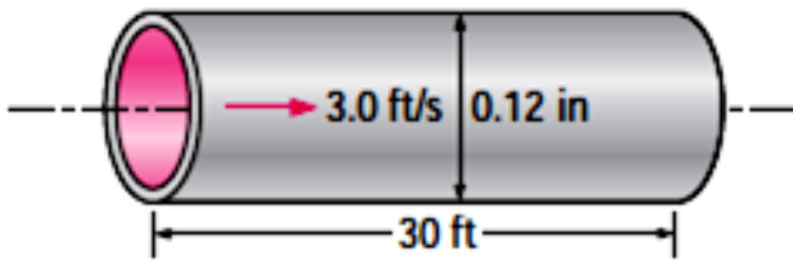
$$V_{avg} = \frac{\dot{V}}{A_c} = \frac{0.00354\ \text{m}^3/\text{s}}{0.001963\ \text{m}^2} = 1.80\ \text{m/s}$$

$$\text{Re} = \frac{\rho V_{avg} D}{\mu} = \frac{(888\ \text{kg/m}^3)(1.80\ \text{m/s})(0.05\ \text{m})}{0.800\ \text{kg/m}\cdot\text{s}} = 100$$

which is much less than 2300. Therefore, the flow is *laminar* for all three cases and the analysis is valid.

EXAMPLE 2. Pressure Drop and Head Loss in a Pipe

- Water at 40∘F (ρ = 62.42 lbm/ft^3 and μ = 1.038 x 10^{-3} lbm/ft · s) is flowing through a 0.12-in (= 0.010 ft) diameter 30-ft-long horizontal pipe steadily at an average velocity of 3.0 ft/s. Determine (a) the head loss, (b) the pressure drop, and (c) the pumping power requirement to overcome this pressure drop.



SOLUTION The average flow velocity in a pipe is given. The head loss, the pressure drop, and the pumping power are to be determined.

Assumptions **1** The flow is steady and incompressible. **2** The entrance effects are negligible, and thus the flow is fully developed. **3** The pipe involves no components such as bends, valves, and connectors.

Properties The density and dynamic viscosity of water are given to be ρ = 62.42 lbm/ft^3 and $\mu = 1.038 \times 10^{-3}$ lbm/ft · s, respectively.

Analysis (*a*) First we need to determine the flow regime. The Reynolds number is

$$\mathrm{Re} = \frac{\rho V_{avg} D}{\mu} = \frac{(62.42\ \mathrm{lbm/ft^3})(3\ \mathrm{ft/s})(0.01\ \mathrm{ft})}{1.038 \times 10^{-3}\ \mathrm{lbm/ft \cdot s}} = 1803$$

which is less than 2300. Therefore, the flow is laminar. Then the friction factor and the head loss become

$$f = \frac{64}{\mathrm{Re}} = \frac{64}{1803} = 0.0355$$

$$h_L = f\frac{L}{D}\frac{V_{avg}^2}{2g} = 0.0355\,\frac{30\ \mathrm{ft}}{0.01\ \mathrm{ft}}\,\frac{(3\ \mathrm{ft/s})^2}{2(32.2\ \mathrm{ft/s^2})} = \mathbf{14.9\ ft}$$

(*b*) Noting that the pipe is horizontal and its diameter is constant, the pressure drop in the pipe is due entirely to the frictional losses and is equivalent to the pressure loss,

$$\Delta P = \Delta P_L = f\frac{L}{D}\frac{\rho V_{avg}^2}{2} = 0.0355\,\frac{30\ \mathrm{ft}}{0.01\ \mathrm{ft}}\,\frac{(62.42\ \mathrm{lbm/ft^3})(3\ \mathrm{ft/s})^2}{2}\left(\frac{1\ \mathrm{lbf}}{32.2\ \mathrm{lbm \cdot ft/s^2}}\right)$$

$$= \mathbf{929\ lbf/ft^2 = 6.45\ psi}$$

(*c*) The volume flow rate and the pumping power requirements are

$$\dot{V} = V_{avg}A_c = V_{avg}(\pi D^2/4) = (3 \text{ ft/s})[\pi(0.01 \text{ ft})^2/4] = 0.000236 \text{ ft}^3/\text{s}$$

$$\dot{W}_{pump} = \dot{V}\,\Delta P = (0.000236 \text{ ft}^3/\text{s})(929 \text{ lbf/ft}^2)\left(\frac{1 \text{ W}}{0.737 \text{ lbf}\cdot\text{ft/s}}\right) = 0.30 \text{ W}$$

Therefore, power input in the amount of 0.30 W is needed to overcome the frictional losses in the flow due to viscosity.

Turbulent Flow in Pipes

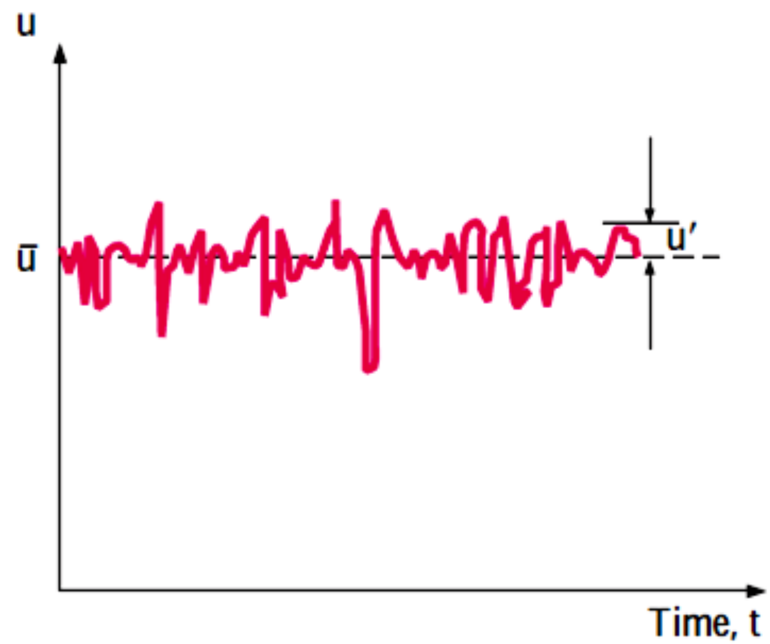
- Most flows encountered in engineering practice are turbulent, and thus it is important to understand how turbulence affects wall shear stress.
- However, turbulent flow is a complex mechanism dominated by fluctuations, and despite tremendous amounts of work done in this area by researchers, the theory of turbulent flow remains largely undeveloped.
- Therefore, we must rely on experiments and the empirical or semi-empirical correlations developed for various situations.
- Turbulent flow is characterized by random and rapid fluctuations of swirling regions of fluid, called **eddies,** throughout the flow. These fluctuations provide an additional mechanism for momentum and energy transfer.

Turbulent Flow in Pipes

- In laminar flow, fluid particles flow in an orderly manner along pathlines, and momentum and energy are transferred across streamlines by molecular diffusion.
- In turbulent flow, the swirling eddies transport mass, momentum, and energy to other regions of flow much more rapidly than molecular diffusion, greatly enhancing mass, momentum, and heat transfer.
- As a result, turbulent flow is associated with much higher values of friction, heat transfer, and mass transfer coefficients.
- The eddy motion in turbulent flow causes significant fluctuations in the values of velocity, temperature, pressure, and even density (in compressible flow).

Turbulent Flow in Pipes

- Fluctuations of the velocity component u with time at a specified location in turbulent flow shown in fig below.



- Instantaneous values of the velocity fluctuate about an average value, which suggests that the velocity can be expressed as the sum of an average value $\overline{u}$ and fluctuating component u’,

$$u = \overline{u} + u'$$

Turbulent Flow in Pipes

Turbulent Shear Stress

- The turbulent shear stress consists of two parts: the laminar component, which accounts for the friction between layers in the flow direction (expressed as $\tau_{lam} = -\mu \; d\bar{u}/dr)$, and the turbulent component, which accounts for the friction between the fluctuating fluid particles and the fluid body (denoted as τ_{turb} and is related to the fluctuation components of velocity).
- Then the total shear stress in turbulent flow can be expressed as

$$\tau_{total} = \tau_{lam} + \tau_{turb}$$

Turbulent Flow in Pipes

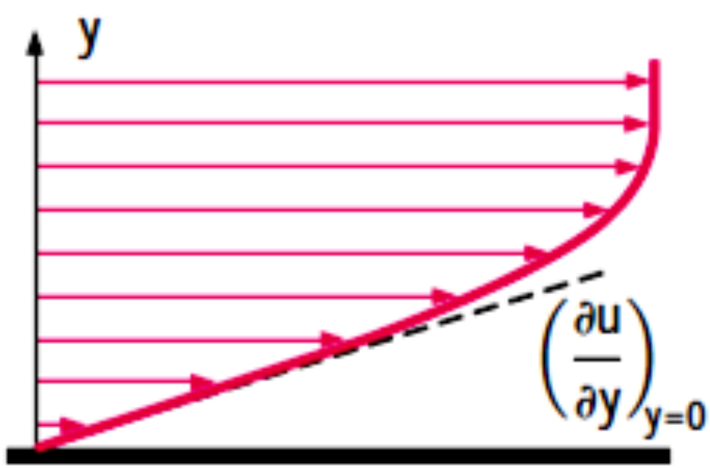
- The total shear stress can be expressed conveniently as

$$\tau_{\text{total}} = (\mu + \mu_t)\frac{\partial \bar{u}}{\partial y} = \rho(\nu + \nu_t)\frac{\partial \bar{u}}{\partial y}$$

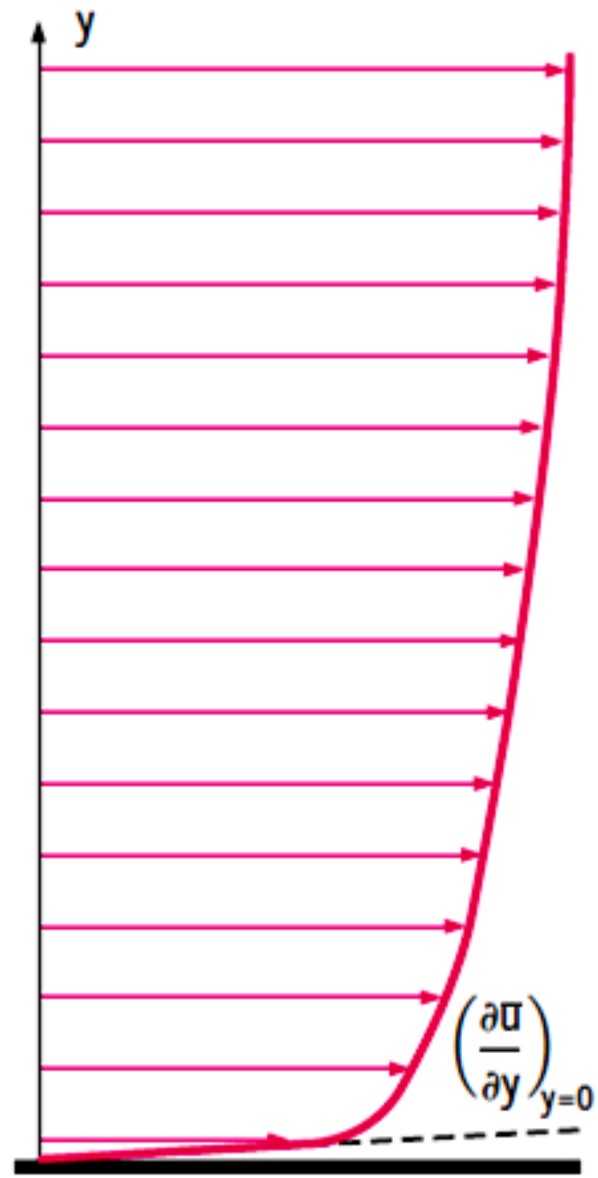
- where μ_t is the **eddy viscosity** or **turbulent viscosity,** which accounts for momentum transport by turbulent eddies.
- $\nu_t = \mu_t/\rho$ is the **kinematic eddy viscosity** or **kinematic turbulent viscosity.**

Turbulent Flow in Pipes

- The velocity gradients at the wall, and thus the wall shear stress, are much larger for turbulent flow than they are for laminar flow.



Laminar flow

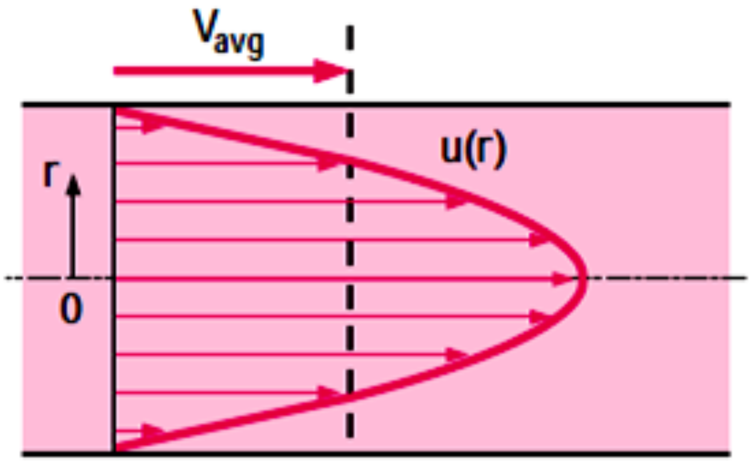


Turbulent flow

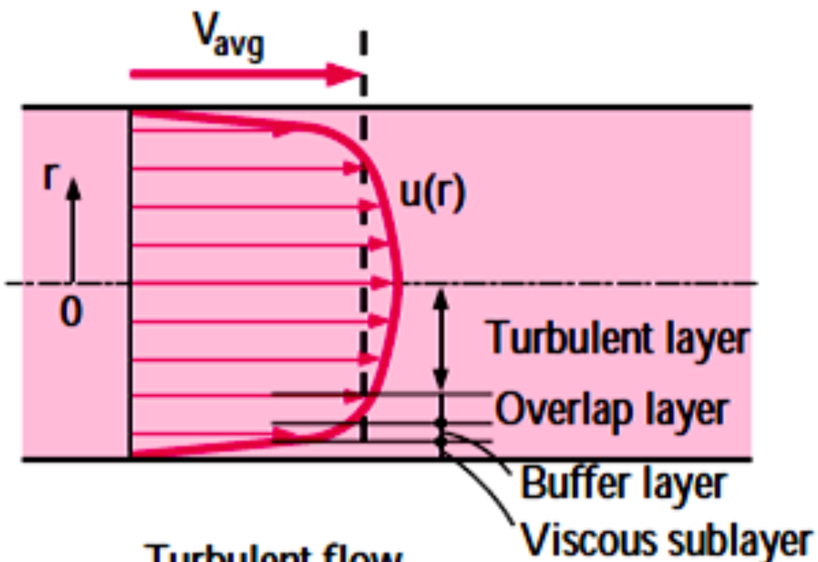
Turbulent Flow in Pipes

Turbulent Velocity Profile

- The velocity profile is parabolic in laminar flow but is much **fuller** in turbulent flow, with a sharp drop near the pipe wall.
- Turbulent flow along a wall can be considered to consist of four regions, characterized by the distance from the wall. The very thin layer next to the wall where viscous effects are dominant is the **viscous (**or **laminar** or **linear** or **wall) sublayer**.



Laminar flow



Turbulent flow

Turbulent Flow in Pipes

- The velocity profile in this layer is very nearly linear, and the flow is streamlined.
- Next to the viscous sublayer is the **buffer layer,** in which turbulent effects are becoming significant, but the flow is still dominated by viscous effects.
- Above the buffer layer is the **overlap (or transition) layer,** also called the **inertial sublayer,** in which the turbulent effects are much more significant, but still not dominant.
- Above that is the **outer (or turbulent) layer in the remaining part of the flow in** which turbulent effects dominate over molecular diffusion (viscous) effects.
- Flow characteristics are quite different in different regions, and thus **it is difficult to come up with an analytic relation for the velocity profile for the entire flow** as we did for laminar flow.

Turbulent Flow in Pipes

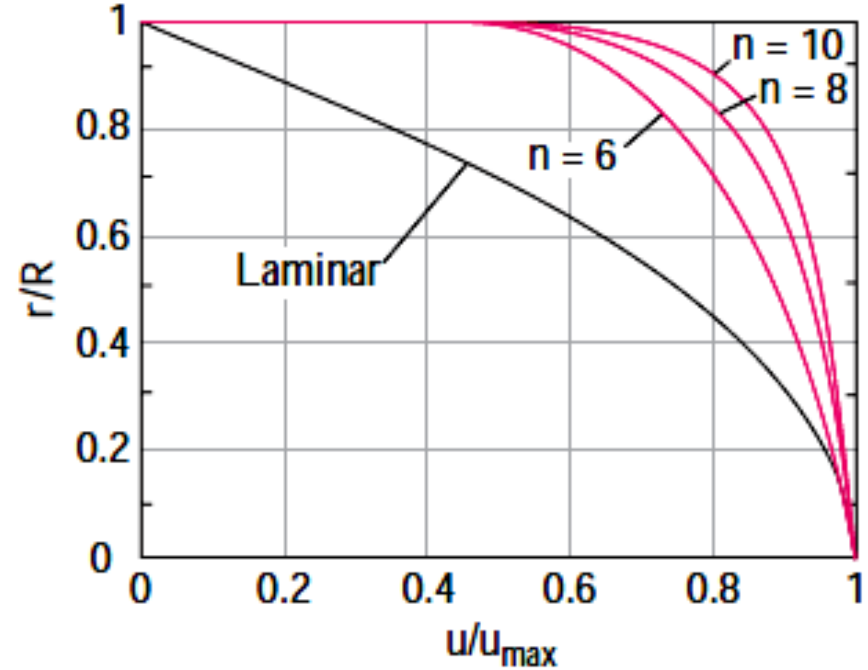
- Numerous empirical velocity profiles exist for turbulent pipe flow. Among those, the simplest and the best known is the **power-law velocity profile** expressed as

Power-law velocity profile: $$\frac{u}{u_{max}} = \left(\frac{y}{R}\right)^{1/n} \quad \text{or} \quad \frac{u}{u_{max}} = \left(1 - \frac{r}{R}\right)^{1/n}$$

- where the exponent n is a constant whose value depends on the Reynolds number. The value of n increases with increasing Reynolds number.
- The value n = 7 generally approximates many flows in practice, giving rise to the term one-seventh power-law velocity profile.

Turbulent Flow in Pipes

- The turbulent velocity profile is fuller than the laminar one, and it becomes more flat as n (and thus the Reynolds number) increases.
- The power-law profile cannot be used to calculate wall shear stress since it gives a velocity gradient of infinity there, and it fails to give zero slope at the centerline.

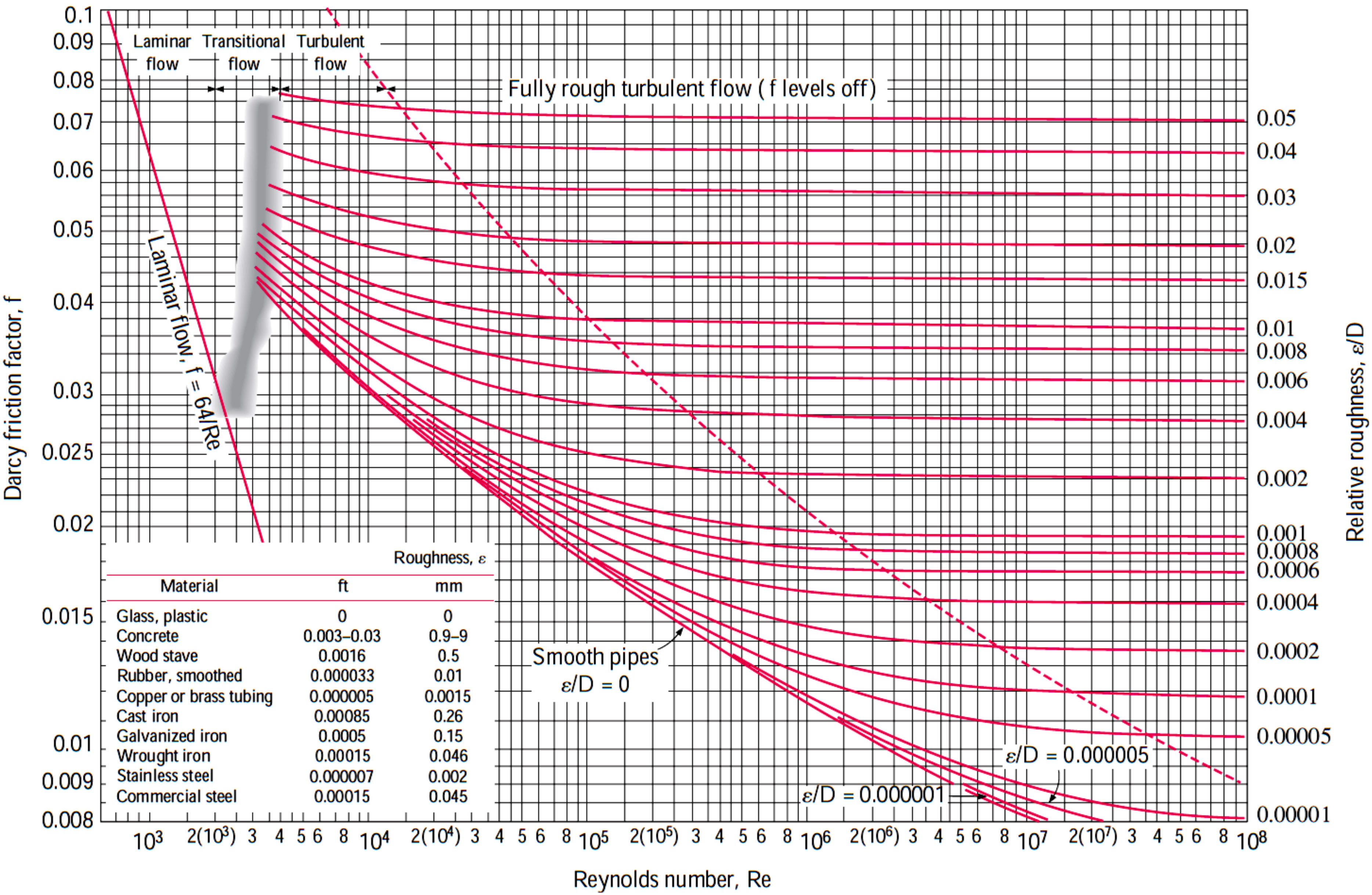


The Moody Chart

- The friction factor in fully developed turbulent pipe flow depends on the Reynolds number and the **relative roughness** ε/D**,** which is the ratio of the mean height of roughness of the pipe to the pipe diameter.
- **Colebrook equation**

$$\frac{1}{\sqrt{f}} = -2.0 \log\left(\frac{\varepsilon/D}{3.7} + \frac{2.51}{Re\sqrt{f}}\right) \qquad \text{(turbulent flow)}$$

- The **Moody chart** presents the Darcy friction factor for pipe flow as a function of the Reynolds number and ε/*D* over a wide range.
- It is probably one of the most widely accepted and used charts in engineering. Although it is developed for circular pipes, it can also be used for noncircular pipes by replacing the diameter by the hydraulic diameter.



	Roughness, ε	
Material	ft	mm
Glass, plastic	0	0
Concrete	0.003–0.03	0.9–9
Wood stave	0.0016	0.5
Rubber, smoothed	0.000033	0.01
Copper or brass tubing	0.000005	0.0015
Cast iron	0.00085	0.26
Galvanized iron	0.0005	0.15
Wrought iron	0.00015	0.046
Stainless steel	0.000007	0.002
Commercial steel	0.00015	0.045

The Moody Chart

- The Moody chart for friction factor for fully developed flow in circular pipes for use in the head loss relation

$$h_L = f\frac{L}{D}\frac{V^2}{2g}.$$

- Friction factors in the turbulent flow are evaluated from the Colebrook equation

$$\frac{1}{\sqrt{f}} = -2\log_{10}\left(\frac{\varepsilon/D}{3.7} + \frac{2.51}{Re\sqrt{f}}\right)$$

The Moody Chart

Equivalent roughness values for new commercial pipes*

Material	Roughness, ε ft	Roughness, ε mm
Glass, plastic	0 (smooth)	
Concrete	0.003–0.03	0.9–9
Wood stave	0.0016	0.5
Rubber, smoothed	0.000033	0.01
Copper or brass tubing	0.000005	0.0015
Cast iron	0.00085	0.26
Galvanized iron	0.0005	0.15
Wrought iron	0.00015	0.046
Stainless steel	0.000007	0.002
Commercial steel	0.00015	0.045

* The uncertainty in these values can be as much as ±60 percent.

Relative Roughness, ε/D	Friction Factor, f
0.0*	0.0119
0.00001	0.0119
0.0001	0.0134
0.0005	0.0172
0.001	0.0199
0.005	0.0305
0.01	0.0380
0.05	0.0716

* Smooth surface. All values are for Re = 10^6 and are calculated from the Colebrook equation.

Turbulent Flow in Pipes

- The Colebrook equation is implicit in f, and thus the determination of the friction factor requires some iteration unless an equation solver is used.
- An approximate explicit relation for f was given by **S. E. Haaland** in 1983 as

$$\frac{1}{\sqrt{f}} \cong -1.8 \log\left[\frac{6.9}{Re} + \left(\frac{\varepsilon/D}{3.7}\right)^{1.11}\right]$$

- The results obtained from this relation are within 2 percent of those obtained from the Colebrook equation.

Turbulent Flow in Pipes

Types of Fluid Flow Problems

- In the design and analysis of piping systems that involve the use of the Moody chart (or the Colebrook equation), we usually encounter three types of problems (the fluid and the roughness of the pipe are assumed to be specified in all cases)
  1. Determining the **pressure drop (or head loss)** when the pipe length and diameter are given for a specified flow rate (or velocity)
  2. Determining the **flow rate** when the pipe length and diameter are given for a specified pressure drop (or head loss)
  3. Determining the **pipe diameter** when the pipe length and flow rate are given for a specified pressure drop (or head loss)

Turbulent Flow in Pipes

- Problems of the first type are straightforward and can be solved **directly by using the Moody chart**.
- Problems of the second type and third type are commonly encountered in engineering design (in the selection of pipe diameter, for example, that minimizes the sum of the construction and pumping costs), but the use of the Moody chart with such problems **requires an iterative approach** unless an equation solver is used.
- To avoid tedious iterations in head loss, flow rate, and diameter calculations, **Swamee and Jain** proposed the following explicit relations in 1976 that are accurate to within 2 percent of the Moody chart:

Turbulent Flow in Pipes

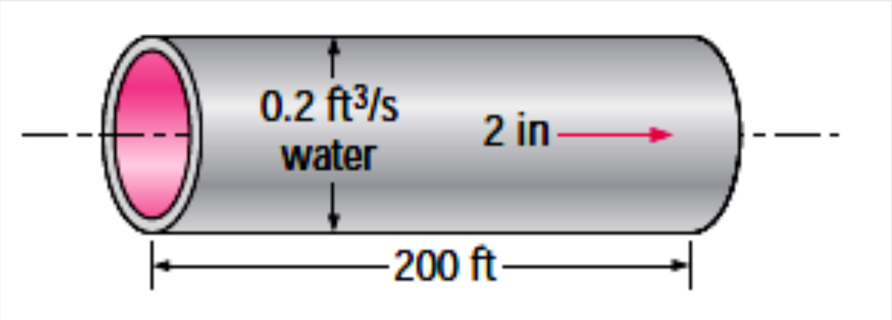
$$h_L = 1.07\frac{\dot{V}^2 L}{gD^5}\left\{\ln\left[\frac{\varepsilon}{3.7D} + 4.62\left(\frac{\nu D}{\dot{V}}\right)^{0.9}\right]\right\}^{-2} \qquad \begin{array}{c} 10^{-6} < \varepsilon/D < 10^{-2} \\ 3000 < Re < 3 \times 10^8 \end{array}$$

$$\dot{V} = -0.965\left(\frac{gD^5 h_L}{L}\right)^{0.5} \ln\left[\frac{\varepsilon}{3.7D} + \left(\frac{3.17\nu^2 L}{gD^3 h_L}\right)^{0.5}\right] \qquad Re > 2000$$

$$D = 0.66\left[\varepsilon^{1.25}\left(\frac{L\dot{V}^2}{gh_L}\right)^{4.75} + \nu\dot{V}^{9.4}\left(\frac{L}{gh_L}\right)^{5.2}\right]^{0.04} \qquad \begin{array}{c} 10^{-6} < \varepsilon/D < 10^{-2} \\ 5000 < Re < 3 \times 10^8 \end{array}$$

EXAMPLE 3. Determining the Head Loss in a Water Pipe

- Water at 60°F (ρ = 62.36 lbm/ft^3 and μ = 7.536 x 10^{-4} lbm/ft · s) is flowing steadily in a 2-in-diameter horizontal pipe made of stainless steel at a rate of 0.2 ft^3/s. Determine the pressure drop, the head loss, and the required pumping power input for flow over a 200-ft-long section of the pipe.



SOLUTION The flow rate through a specified water pipe is given. The pressure drop, the head loss, and the pumping power requirements are to be determined.

Assumptions **1** The flow is steady and incompressible. **2** The entrance effects are negligible, and thus the flow is fully developed. **3** The pipe involves no components such as bends, valves, and connectors. **4** The piping section involves no work devices such as a pump or a turbine.

Properties The density and dynamic viscosity of water are given to be $\rho = 62.36\ \text{lbm/ft}^3$ and $\mu = 7.536 \times 10^{-4}\ \text{lbm/ft} \cdot \text{s}$, respectively.

Analysis We recognize this as a problem of the first type, since flow rate, pipe length, and pipe diameter are known. First we calculate the average velocity and the Reynolds number to determine the flow regime:

$$V = \frac{\dot{V}}{A_c} = \frac{\dot{V}}{\pi D^2/4} = \frac{0.2\ \text{ft}^3/\text{s}}{\pi(2/12\ \text{ft})^2/4} = 9.17\ \text{ft/s}$$

$$\text{Re} = \frac{\rho V D}{\mu} = \frac{(62.36\ \text{lbm/ft}^3)(9.17\ \text{ft/s})(2/12\ \text{ft})}{7.536 \times 10^{-4}\ \text{lbm/ft} \cdot \text{s}} = 126{,}400$$

which is greater than 4000. Therefore, the flow is turbulent. The relative roughness of the pipe is calculated using the Table

$$\varepsilon/D = \frac{0.000007 \text{ ft}}{2/12 \text{ ft}} = 0.000042$$

The friction factor corresponding to this relative roughness and the Reynolds number can simply be determined from the Moody chart. To avoid any reading error, we determine *f* from the Colebrook equation:

$$\frac{1}{\sqrt{f}} = -2.0 \log\left(\frac{\varepsilon/D}{3.7} + \frac{2.51}{\text{Re}\sqrt{f}}\right) \rightarrow \frac{1}{\sqrt{f}} = -2.0 \log\left(\frac{0.000042}{3.7} + \frac{2.51}{126{,}400\sqrt{f}}\right)$$

Using an equation solver or an iterative scheme, the friction factor is determined to be $f = 0.0174$. Then the pressure drop (which is equivalent to pressure loss in this case), head loss, and the required power input become

$$\Delta P = \Delta P_L = f\frac{L}{D}\frac{\rho V^2}{2} = 0.0174\,\frac{200 \text{ ft}}{2/12 \text{ ft}}\,\frac{(62.36 \text{ lbm/ft}^3)(9.17 \text{ ft/s})^2}{2}\left(\frac{1 \text{ lbf}}{32.2 \text{ lbm}\cdot\text{ft/s}^2}\right)$$

$$= \mathbf{1700 \text{ lbf/ft}^2} = \mathbf{11.8 \text{ psi}}$$

$$h_L = \frac{\Delta P_L}{\rho g} = f\frac{L}{D}\frac{V^2}{2g} = 0.0174\,\frac{200 \text{ ft}}{2/12 \text{ ft}}\,\frac{(9.17 \text{ ft/s})^2}{2(32.2 \text{ ft/s}^2)} = \mathbf{27.3 \text{ ft}}$$

$$\dot{W}_{pump} = \dot{V}\,\Delta P = (0.2 \text{ ft}^3/\text{s})(1700 \text{ lbf/ft}^2)\left(\frac{1 \text{ W}}{0.737 \text{ lbf}\cdot\text{ft/s}}\right) = \mathbf{461 \text{ W}}$$

Therefore, power input in the amount of 461 W is needed to overcome the frictional losses in the pipe.

- The friction factor could also be determined easily from the explicit Haaland relation. It would give $f = 0.0172$, which is sufficiently close to 0.0174.

Minor Losses

- The fluid in a typical piping system passes through various fittings, valves, bends, elbows, tees, inlets, exits, enlargements, and contractions in addition to the pipes.
- These components interrupt the smooth flow of the fluid and cause additional losses because of the flow separation and mixing they induce.
- In a typical system with long pipes, these losses are minor compared to the total head loss in the pipes (the **major losses)** and are called **minor losses.**
- Although this is generally true, in some cases the minor losses may be greater than the major losses. This is the case, for example, in systems with several turns and valves in a short distance.

Minor Losses

- The head loss introduced by a completely open valve, for example, may be negligible. But a partially closed valve may cause the largest head loss in the system, as evidenced by the drop in the flow rate.
- Flow through valves and fittings is very complex, and a theoretical analysis is generally not plausible.
- Therefore, minor losses are determined experimentally, usually by the manufacturers of the components.
- Minor losses are usually expressed in terms of the **loss coefficient** K_L (also called the **resistance coefficient),** defined as

Loss coefficient: $$K_L = \frac{h_L}{V^2/(2g)}$$

- where h_L is the additional irreversible head loss in the piping system caused by insertion of the component, and is defined as $h_L = \Delta PL/\rho g$.

Minor Losses

- Once all the loss coefficients are available, the total head loss in a piping system is determined from

Total head loss (general): $$h_{L,\,total} = h_{L,\,major} + h_{L,\,minor} = \sum_i f_i \frac{L_i}{D_i}\frac{V_i^2}{2g} + \sum_j K_{L,j}\frac{V_j^2}{2g}$$

- where i represents each pipe section with constant diameter and j represents each component that causes a minor loss.
- If the entire piping system being analyzed has a constant diameter

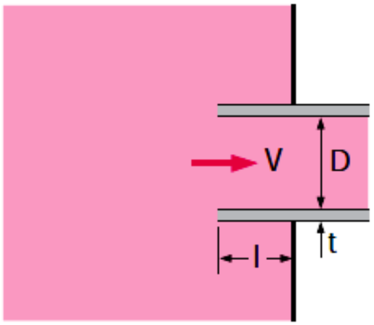
Total head loss (D = constant): $$h_{L,\,total} = \left(f\frac{L}{D} + \sum K_L\right)\frac{V^2}{2g}$$

- where V is the average flow velocity through the entire system (note that V = constant since D = constant).

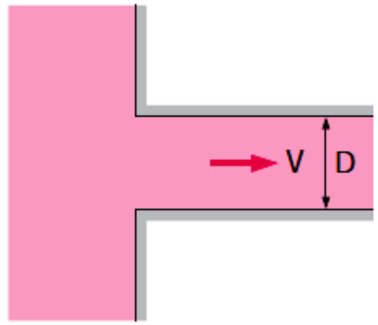
Loss coefficients K_L of various pipe components for turbulent flow (for use in the relation $h_L = K_L V^2/(2g)$, where V is the average velocity in the pipe that contains the component)*

Pipe Inlet

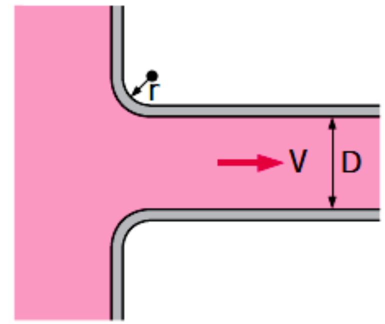
Reentrant: $K_L = 0.80$
($t \ll D$ and $l \approx 0.1D$)



Sharp-edged: $K_L = 0.50$

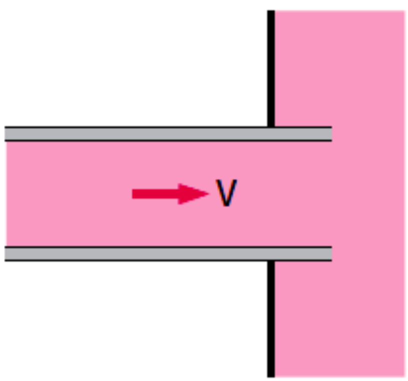


Well-rounded ($r/D > 0.2$): $K_L = 0.03$
Slightly rounded ($r/D = 0.1$): $K_L = 0.12$
(see Fig. 8–36)

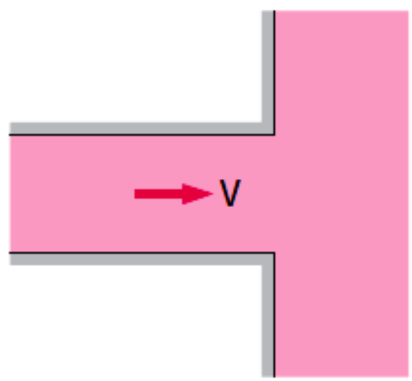


Pipe Exit

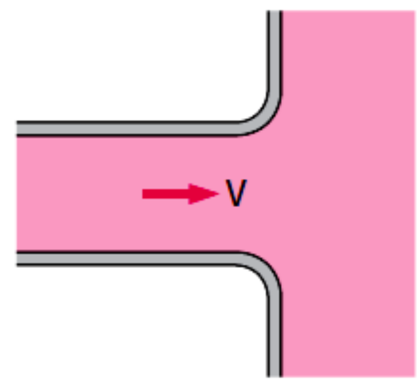
Reentrant: $K_L = \alpha$



Sharp-edged: $K_L = \alpha$



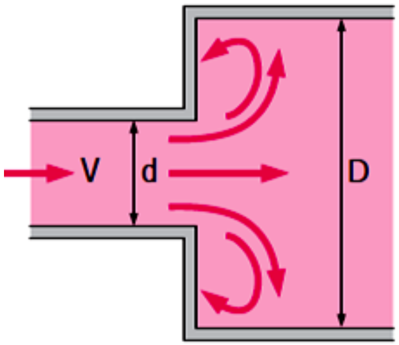
Rounded: $K_L = \alpha$



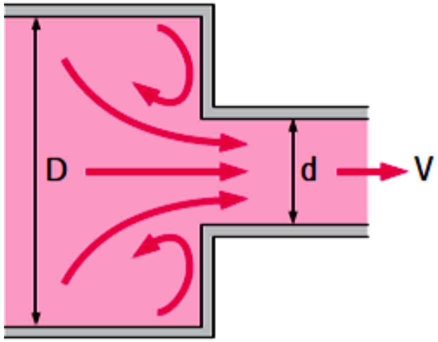
Note: The kinetic energy correction factor is $\alpha = 2$ for fully developed laminar flow, and $\alpha \approx 1$ for fully developed turbulent flow.

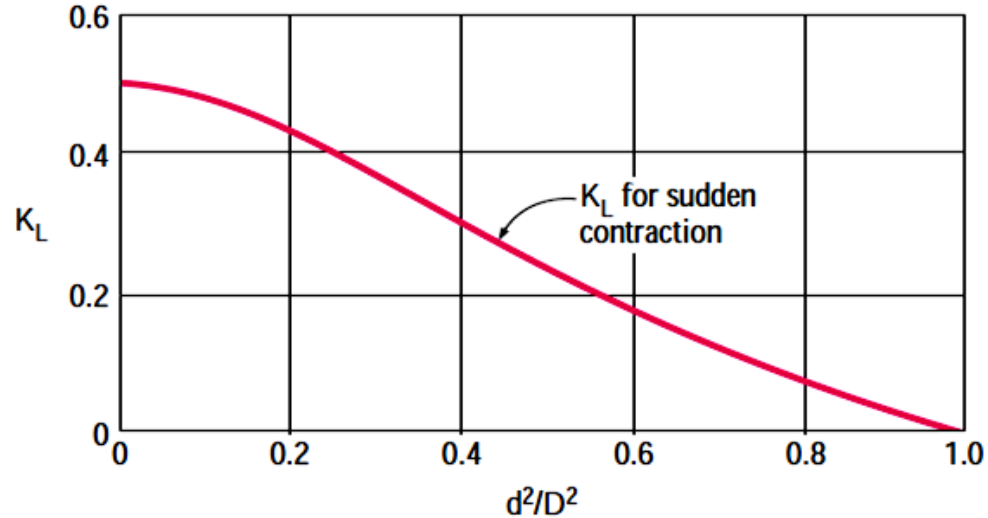
Sudden Expansion and Contraction (based on the velocity in the smaller-diameter pipe)

Sudden expansion: $K_L = \left(1 - \dfrac{d^2}{D^2}\right)^2$



Sudden contraction: See chart.





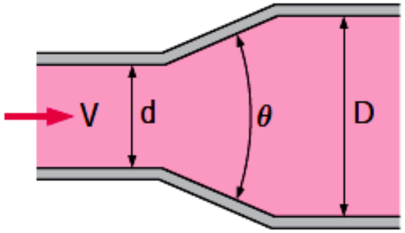
Gradual Expansion and Contraction (based on the velocity in the smaller-diameter pipe)

Expansion:

$K_L = 0.02$ for $\theta = 20°$

$K_L = 0.04$ for $\theta = 45°$

$K_L = 0.07$ for $\theta = 60°$



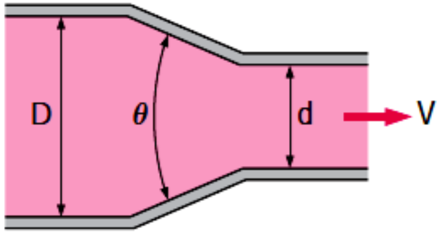
Contraction (for $\theta = 20°$):

$K_L = 0.30$ for $d/D = 0.2$

$K_L = 0.25$ for $d/D = 0.4$

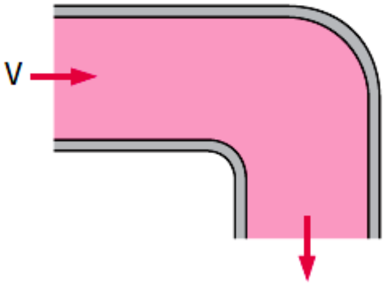
$K_L = 0.15$ for $d/D = 0.6$

$K_L = 0.10$ for $d/D = 0.8$

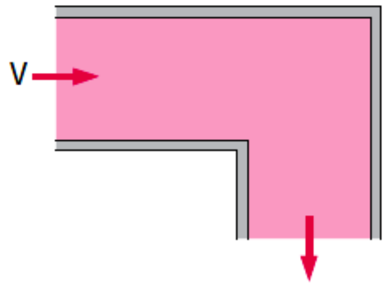


Bends and Branches

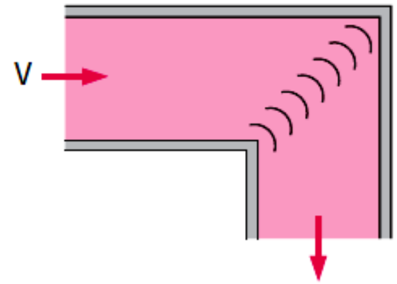
90° smooth bend:
Flanged: $K_L = 0.3$
Threaded: $K_L = 0.9$



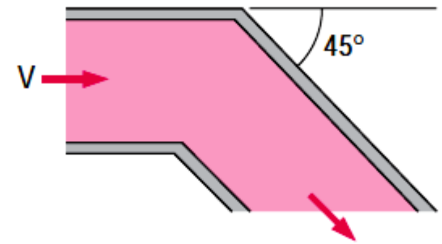
90° miter bend
(without vanes): $K_L = 1.1$



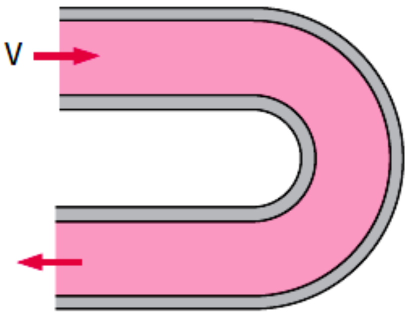
90° miter bend
(with vanes): $K_L = 0.2$



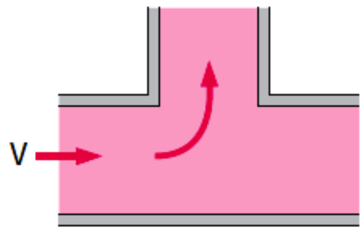
45° threaded elbow:
$K_L = 0.4$



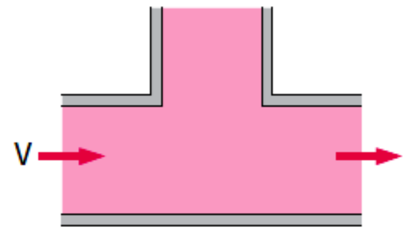
180° return bend:
Flanged: $K_L = 0.2$
Threaded: $K_L = 1.5$



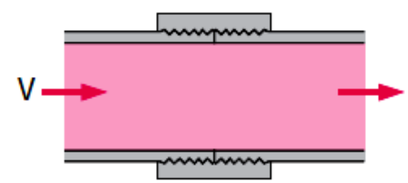
Tee (branch flow):
Flanged: $K_L = 1.0$
Threaded: $K_L = 2.0$



Tee (line flow):
Flanged: $K_L = 0.2$
Threaded: $K_L = 0.9$



Threaded union:
$K_L = 0.08$



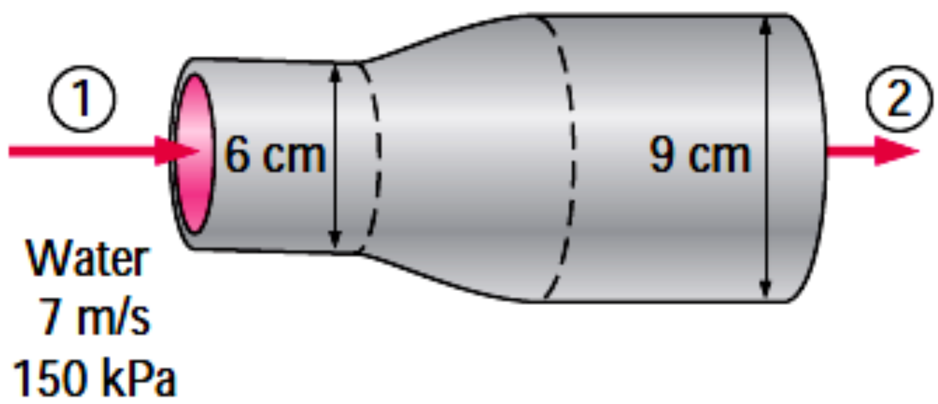
Valves

Globe valve, fully open: $K_L = 10$
Angle valve, fully open: $K_L = 5$
Ball valve, fully open: $K_L = 0.05$
Swing check valve: $K_L = 2$

Gate valve, fully open: $K_L = 0.2$
$\frac{1}{4}$ closed: $K_L = 0.3$
$\frac{1}{2}$ closed: $K_L = 2.1$
$\frac{3}{4}$ closed: $K_L = 17$

EXAMPLE 4. Head Loss and Pressure Rise during Gradual Expansion

- A 6-cm-diameter horizontal water pipe expands gradually to a 9-cm-diameter pipe . The walls of the expansion section are angled 30◦ from the horizontal. The average velocity and pressure of water before the expansion section are 7 m/s and 150 kPa, respectively. Determine the head loss in the expansion section and the pressure in the larger-diameter pipe.   Ans. **0.175m, 168 KPa**



SOLUTION A horizontal water pipe expands gradually into a larger-diameter pipe. The head loss and pressure after the expansion are to be determined.

Assumptions **1** The flow is steady and incompressible. **2** The flow at sections 1 and 2 is fully developed and turbulent with $\alpha_1 = \alpha_2 \cong 1.06$.

$\rightarrow \alpha_1$ and α_2 are kinetic energy correction factors

Properties We take the density of water to be $\rho = 1000\ \text{kg/m}^3$. The loss coefficient for gradual expansion of $\theta = 60°$ total included angle is $K_L = 0.07$.

Analysis Noting that the density of water remains constant, the downstream velocity of water is determined from conservation of mass to be

$$\dot{m}_1 = \dot{m}_2 \quad \rightarrow \quad \rho V_1 A_1 = \rho V_2 A_2 \quad \rightarrow \quad V_2 = \frac{A_1}{A_2} V_1 = \frac{D_1^2}{D_2^2} V_1$$

$$V_2 = \frac{(0.06\ \text{m})^2}{(0.09\ \text{m})^2}(7\ \text{m/s}) = 3.11\ \text{m/s}$$

Then the irreversible head loss in the expansion section becomes

$$h_L = K_L \frac{V_1^2}{2g} = (0.07)\frac{(7\ \text{m/s})^2}{2(9.81\ \text{m/s}^2)} = 0.175\ \text{m}$$

Noting that $z_1 = z_2$ and there are no pumps or turbines involved, the energy equation for the expansion section can be expressed in terms of heads as

$$\frac{P_1}{\rho g} + \alpha_1 \frac{V_1^2}{2g} + \cancel{z_1} + h_{\text{pump, u}}^{\nearrow 0} = \frac{P_2}{\rho g} + \alpha_2 \frac{V_2^2}{2g} + \cancel{z_2} + h_{\text{turbine, e}}^{\nearrow 0} + h_L$$

$$\rightarrow \frac{P_1}{\rho g} + \alpha_1 \frac{V_1^2}{2g} = \frac{P_2}{\rho g} + \alpha_2 \frac{V_2^2}{2g} + h_L$$

Solving for P_2 and substituting,

$$P_2 = P_1 + \rho\left\{\frac{\alpha_1 V_1^2 - \alpha_2 V_2^2}{2} - gh_L\right\} = (150 \text{ kPa}) + (1000 \text{ kg/m}^3)$$

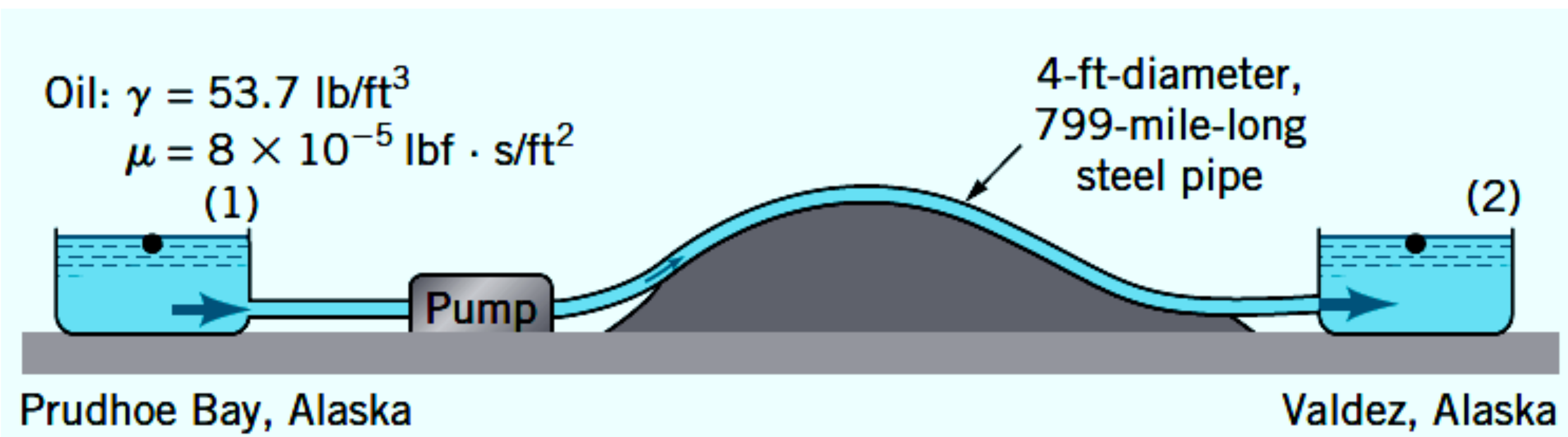
$$\times \left\{\frac{1.06(7 \text{ m/s})^2 - 1.06(3.11 \text{ m/s})^2}{2} - (9.81 \text{ m/s}^2)(0.175 \text{ m})\right\}$$

$$\times \left(\frac{1 \text{ kN}}{1000 \text{ kg} \cdot \text{m/s}}\right)\left(\frac{1 \text{ kPa}}{1 \text{ kN/m}^2}\right)$$

$$= 169 \text{ kPa}$$

Example 5. Determine Head Loss

- As shown in Fig. below, crude oil at 140 ◦F with γ =53.7 lb/ft^3 and $\mu = 8 \times 10^5$ lb . s ft^2 (about four times the viscosity of water) is pumped across Alaska through the Alaskan pipeline, a 799-mile-long, 4-ft-diameter steel pipe, at a maximum rate of Q = 2.4 million barrels day = 117 ft^3 /s. Determine the horsepower needed for the pumps that drive this large system.



Solution

- From the energy equation we obtain

$$\frac{p_1}{\gamma} + \frac{V_1^2}{2g} + z_1 + h_p = \frac{p_2}{\gamma} + \frac{V_2^2}{2g} + z_2 + h_L$$

where points (1) and (2) represent locations within the large holding tanks at either end of the line and h_p is the head provided to the oil by the pumps. We assume that $z_1 = z_2$ (pumped from sea level to sea level), $p_1 = p_2 = V_1 = V_2 = 0$ (large, open tanks) and $h_L = (f\ell/D)V^2/2g$. Minor losses are negligible because of the large length-to-diameter ratio of the relatively straight, uninterrupted pipe; $\ell/D = (799 \text{ mi}) \times (5280 \text{ ft/mi})/(4 \text{ ft}) = 1.05 \times 10^6$. Thus,

$$h_p = h_L = f\frac{\ell}{D}\frac{V^2}{2g}$$

where $V = Q/A = (117\ \text{ft}^3/\text{s})/[\pi(4\ \text{ft})^2/4] = 9.31\ \text{ft/s}$.

$f = 0.0125$ since $\varepsilon/D = (0.00015\ \text{ft})/(4\ \text{ft}) = 0.0000375$

and $\text{Re} = \rho VD/\mu$

$= [(53.7/32.2)\ \text{slugs/ft}^3]\ (9.31\ \text{ft/s})(4.0\ \text{ft})/(8 \times 10^{-5}\ \text{lb}\cdot\text{s/ft}^2) = 7.76 \times 10^5$.

Thus,

$$h_p = 0.0125(1.05 \times 10^6)\frac{(9.31\ \text{ft/s})^2}{2(32.2\ \text{ft/s}^2)} = 17{,}700\ \text{ft}$$

and the actual power supplied to the fluid, $\mathcal{P}_a$, is

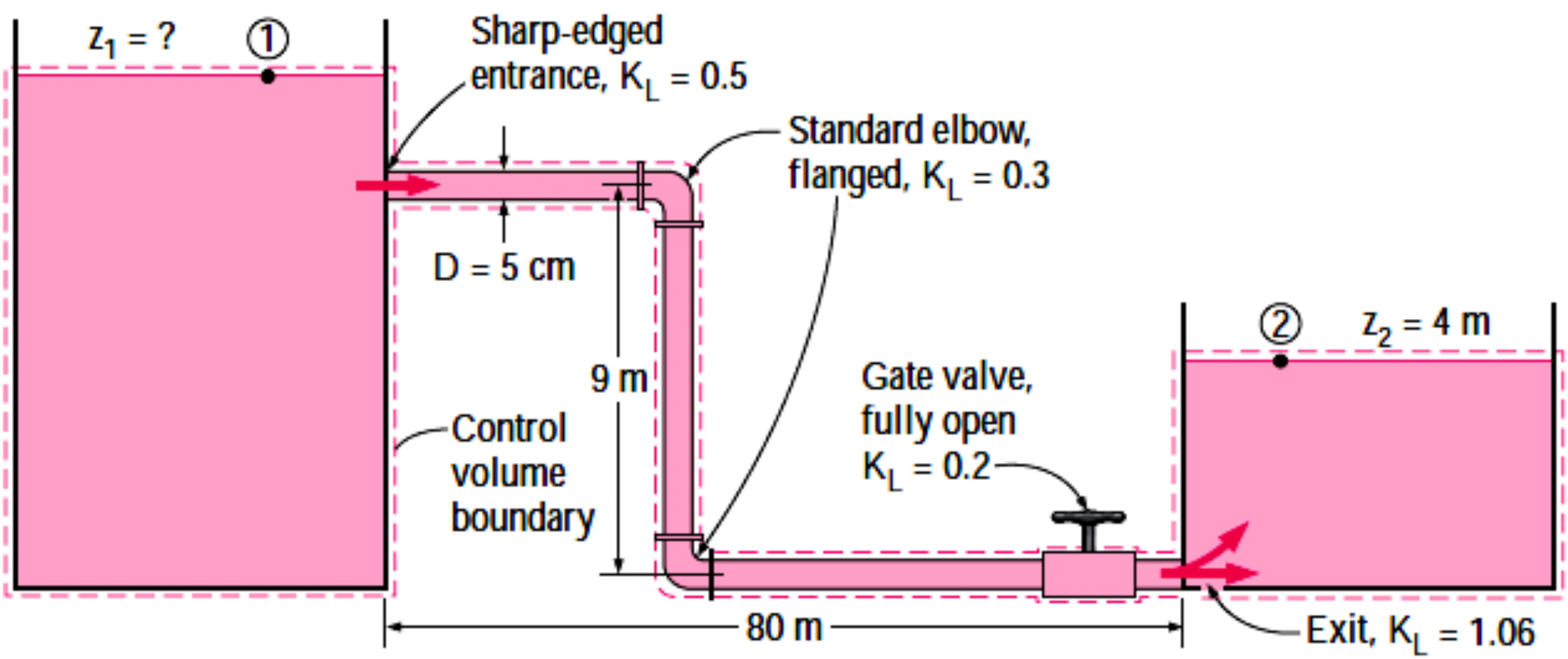
$$\mathcal{P}_a = \gamma Q h_p = (53.7\ \text{lb/ft}^3)(117\ \text{ft}^3/\text{s})(17{,}700\ \text{ft})$$

$$= 1.11 \times 10^8\ \text{ft}\cdot\text{lb/s}\left(\frac{1\ \text{hp}}{550\ \text{ft}\cdot\text{lb/s}}\right)$$

$$= 202{,}000\ \text{hp}$$ **(Ans)**

Example 6. Minor losses

- Water at 10◦C flows from a large reservoir to a smaller one through a 5-cm diameter cast iron piping system, as shown in Fig. below. Determine the elevation z_1 for a flow rate of 6 L/s.



SOLUTION The flow rate through a piping system connecting two reservoirs is given. The elevation of the source is to be determined.

Assumptions **1** The flow is steady and incompressible. **2** The elevations of the reservoirs remain constant. **3** There are no pumps or turbines in the line.

Properties The density and dynamic viscosity of water at 10°C are $\rho = 999.7\ \text{kg/m}^3$ and $\mu = 1.307 \times 10^{-3}\ \text{kg/m}\cdot\text{s}$. The roughness of cast iron pipe is $\varepsilon = 0.00026$ m.

Analysis The piping system involves 89 m of piping, a sharp-edged entrance ($K_L = 0.5$), two standard flanged elbows ($K_L = 0.3$ each), a fully open gate valve ($K_L = 0.2$), and a submerged exit ($K_L = 1.06$). We choose points 1 and 2 at the free surfaces of the two reservoirs. Noting that the fluid at both points is open to the atmosphere (and thus $P_1 = P_2 = P_{atm}$) and that the fluid velocities at both points are zero ($V_1 = V_2 = 0$), the energy equation for a control volume between these two points simplifies to

$$\cancel{\frac{P_1}{\rho g}} + \alpha_1 \cancelto{0}{\frac{V_1^2}{2g}} + z_1 = \cancel{\frac{P_2}{\rho g}} + \alpha_2 \cancelto{0}{\frac{V_2^2}{2g}} + z_2 + h_L \quad \rightarrow \quad z_1 = z_2 + h_L$$

where

$$h_L = h_{L,\,total} = h_{L,\,major} + h_{L,\,minor} = \left(f\frac{L}{D} + \sum K_L\right)\frac{V^2}{2g}$$

since the diameter of the piping system is constant. The average velocity in the pipe and the Reynolds number are

$$V = \frac{\dot{V}}{A_c} = \frac{\dot{V}}{\pi D^2/4} = \frac{0.006 \text{ m}^3/\text{s}}{\pi(0.05 \text{ m})^2/4} = 3.06 \text{ m/s}$$

$$\text{Re} = \frac{\rho V D}{\mu} = \frac{(999.7 \text{ kg/m}^3)(3.06 \text{ m/s})(0.05 \text{ m})}{1.307 \times 10^{-3} \text{ kg/m}\cdot\text{s}} = 117{,}000$$

The flow is turbulent since Re > 4000. Noting that $\varepsilon/D = 0.00026/0.05 = 0.0052$, the friction factor can be determined from the Colebrook equation (or the Moody chart),

$$\frac{1}{\sqrt{f}} = -2.0 \log\left(\frac{\varepsilon/D}{3.7} + \frac{2.51}{\text{Re}\sqrt{f}}\right) \quad \rightarrow \quad \frac{1}{\sqrt{f}} = -2.0 \log\left(\frac{0.0052}{3.7} + \frac{2.51}{117{,}000\sqrt{f}}\right)$$

It gives $f = 0.0315$. The sum of the loss coefficients is

$$\sum K_L = K_{L,\text{ entrance}} + 2K_{L,\text{ elbow}} + K_{L,\text{ valve}} + K_{L,\text{ exit}}$$
$$= 0.5 + 2 \times 0.3 + 0.2 + 1.06 = 2.36$$

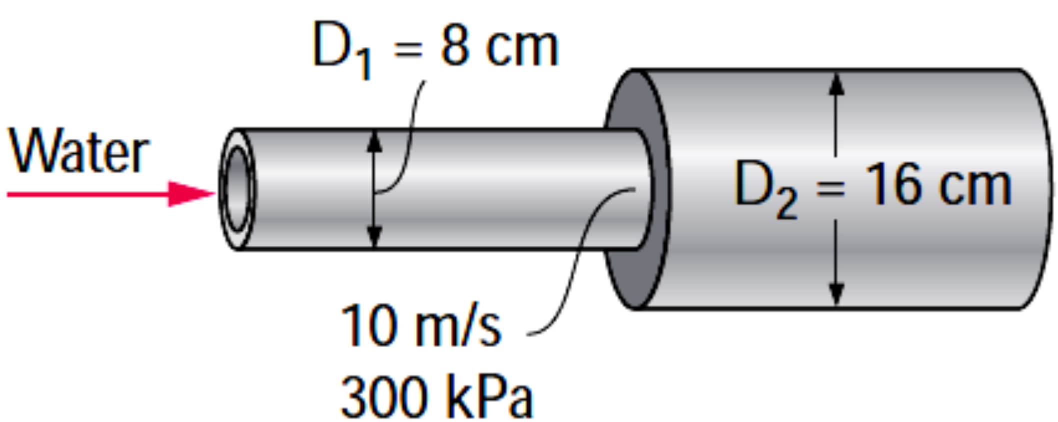
Then the total head loss and the elevation of the source become

$$h_L = \left(f\frac{L}{D} + \sum K_L\right)\frac{V^2}{2g} = \left(0.0315\frac{89 \text{ m}}{0.05 \text{ m}} + 2.36\right)\frac{(3.06 \text{ m/s})^2}{2(9.81 \text{ m/s}^2)} = 27.9 \text{ m}$$

$$z_1 = z_2 + h_L = 4 + 27.9 = 31.9 \text{ m}$$

Example 7

- A horizontal pipe has an abrupt expansion from $D_1 = 8$ cm to $D_2 = 16$ cm. The water velocity in the smaller section is 10 m/s and the flow is turbulent. The pressure in the smaller section is $P_1 = 300$ kPa. Taking the kinetic energy correction factor to be 1.06 at both the inlet and the outlet, determine the downstream pressure P_2, and estimate the error that would have occurred if Bernoulli's equation had been used.



Solution A horizontal water pipe has an abrupt expansion. The water velocity and pressure in the smaller diameter pipe are given. The pressure after the expansion and the error that would have occurred if the Bernoulli Equation had been used are to be determined.

Assumptions **1** The flow is steady, horizontal, and incompressible. **2** The flow at both the inlet and the outlet is fully developed and turbulent with kinetic energy corrections factors of $\alpha_1 = \alpha_2 = 1.06$ (given).

Properties We take the density of water to be $\rho = 1000$ kg/m^3.

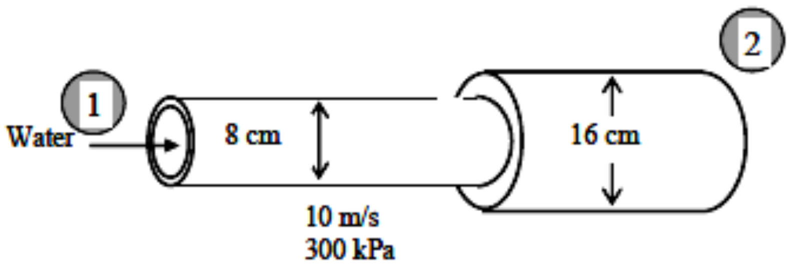
Analysis Noting that ρ = const. (incompressible flow), the downstream velocity of water is

$$\dot{m}_1 = \dot{m}_2 \;\rightarrow\; \rho V_1 A_1 = \rho V_2 A_2 \;\rightarrow\; V_2 = \frac{A_1}{A_2}V_1 = \frac{\pi D_1^2/4}{\pi D_2^2/4}V_1 = \frac{D_1^2}{D_2^2}V_1 = \frac{(0.08\text{ m})^2}{(0.16\text{ m})^2}(10\text{ m/s}) = 2.5\text{ m/s}$$

The loss coefficient for sudden expansion and the head loss can be calculated from

$$K_L = \left(1-\frac{A_{\text{small}}}{A_{\text{large}}}\right)^2 = \left(1-\frac{D_1^2}{D_2^2}\right)^2 = \left(1-\frac{0.08^2}{0.16^2}\right)^2 = 0.5625$$

$$h_L = K_L\frac{V_1^2}{2g} = (0.5625)\frac{(10\text{ m/s})^2}{2(9.81\text{ m/s}^2)} = 2.87\text{ m}$$



Noting that $z_1 = z_2$ and there are no pumps or turbines involved, the energy equation for the expansion section can be expressed in terms of heads as

$$\frac{P_1}{\rho g}+\alpha_1\frac{V_1^2}{2g}+z_1+h_{\text{pump,u}} = \frac{P_2}{\rho g}+\alpha_2\frac{V_2^2}{2g}+z_2+h_{\text{turbine,e}}+h_L \quad\rightarrow\quad \frac{P_1}{\rho g}+\alpha_1\frac{V_1^2}{2g} = \frac{P_2}{\rho g}+\alpha_2\frac{V_2^2}{2g}+h_L$$

Solving for P_2 and substituting,

$$P_2 = P_1 + \rho\left\{\frac{\alpha_1 V_1^2 - \alpha_2 V_2^2}{2} - g h_L\right\}$$

$$= (300\text{ kPa}) + (1000\text{ kg/m}^3)\left\{\frac{1.06(10\text{ m/s})^2 - 1.06(2.5\text{ m/s})^2}{2} - (9.81\text{ m/s}^2)(2.87\text{ m})\right\}\left(\frac{1\text{ kN}}{1000\text{ kg}\cdot\text{m/s}}\right)\left(\frac{1\text{ kPa}}{1\text{ kN/m}^2}\right)$$

$$= \mathbf{322\ kPa}$$

Therefore, despite the head (and pressure) loss, the pressure increases from 300 kPa to 321 kPa after the expansion. This is due to the conversion of dynamic pressure to static pressure when the velocity is decreased.

When the head loss is disregarded, the downstream pressure is determined from the Bernoulli equation to be

$$\frac{P_1}{\rho g}+\frac{V_1^2}{2g}+z_1=\frac{P_2}{\rho g}+\frac{V_2^2}{2g}+z_2 \quad \rightarrow \quad \frac{P_1}{\rho g}+\frac{V_1^2}{2g}=\frac{P_2}{\rho g}+\frac{V_2^2}{2g} \quad \rightarrow \quad P_1=P_1+\rho\frac{V_1{}^2-V_2{}^2}{2}$$

Substituting,

$$P_2=(300\text{ kPa})+(1000\text{ kg/m}^3)\frac{(10\text{ m/s})^2-(2.5\text{ m/s})^2}{2}\left(\frac{1\text{ kN}}{1000\text{ kg}\cdot\text{m/s}}\right)\left(\frac{1\text{ kPa}}{1\text{ kN/m}^2}\right)=347\text{ kPa}$$

Therefore, the error in the Bernoulli equation is $\quad \text{Error} = P_{2,\text{ Bernoulli}} - P_2 \; = 347 - 322 = \mathbf{25.0\ kPa}$

Note that the use of the Bernoulli equation results in an error of (347 – 322) / 322 = 0.078 or 7.8%.

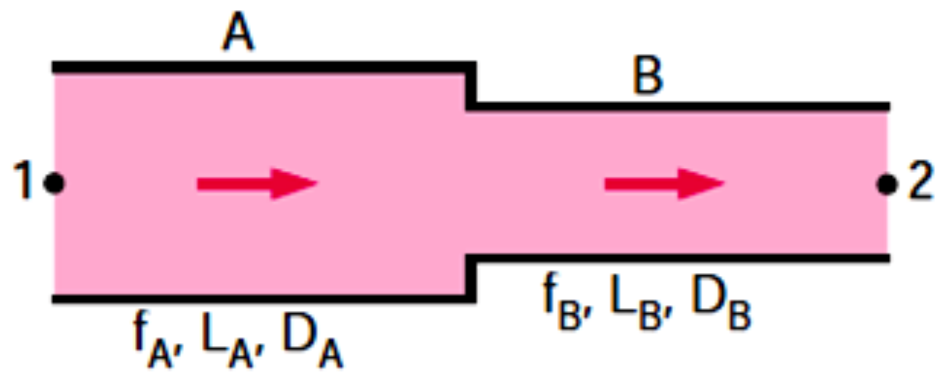
PIPING NETWORKS

- Most piping systems encountered in practice such as the water distribution systems in cities or commercial or residential establishments involve numerous parallel and series connections.

Pipes in Series

- When the pipes are connected **in series,** the flow rate through the entire system remains constant regardless of the diameters of the individual pipes in the system. This is a natural consequence of the conservation of mass principle for steady incompressible flow.
- The total head loss in this case is equal to the sum of the head losses in individual pipes in the system, including the minor losses.

PIPING NETWORKS

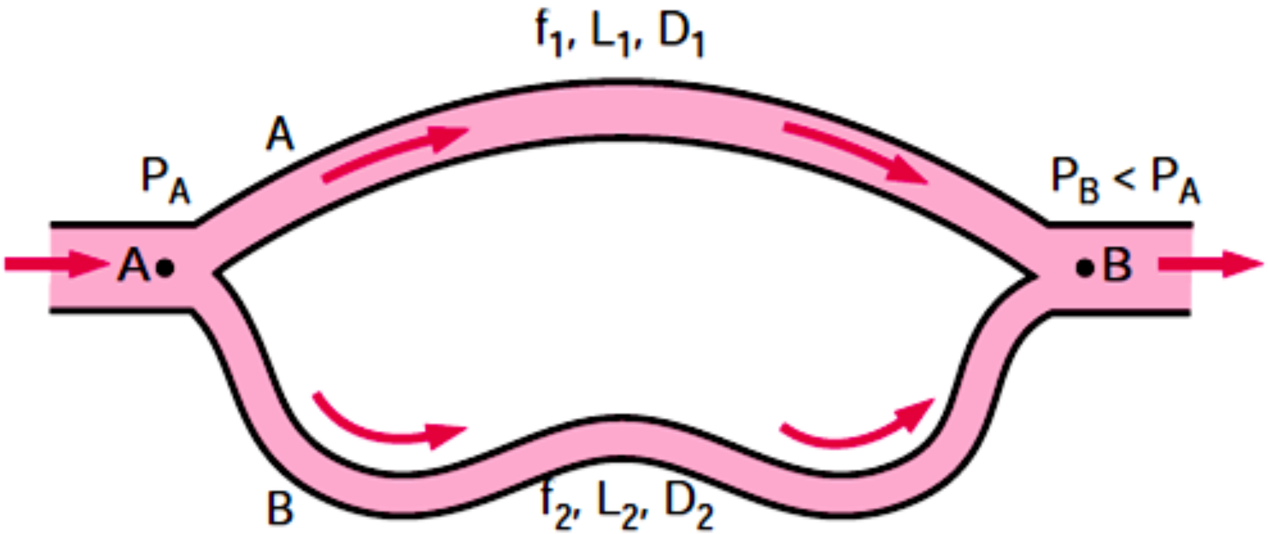


$$\dot{V}_A = \dot{V}_B$$

$$h_{L,\,1\text{-}2} = h_{L,\,A} + h_{L,\,B}$$

- For a pipe that branches out into two (or more) **parallel pipes** and then rejoins at a junction downstream, the total flow rate is the sum of the flow rates in the individual pipes. The pressure drop (or head loss) in each individual pipe connected in parallel must be the same since $\Delta P = P_A - P_B$ *and* the junction pressures P_A and P_B are the same for all the individual pipes.

PIPING NETWORKS



$$h_{L,1} = h_{L,2}$$
$$\dot{V}_A = \dot{V}_1 + \dot{V}_2 = \dot{V}_B$$

- For a system of two parallel pipes 1 and 2 between junctions A and B with negligible minor losses, this can be expressed as

$$h_{L,1} = h_{L,2} \quad \rightarrow \quad f_1 \frac{L_1}{D_1} \frac{V_1^2}{2g} = f_2 \frac{L_2}{D_2} \frac{V_2^2}{2g}$$

PIPING NETWORKS

Then the ratio of the average velocities and the flow rates in the two parallel pipes become

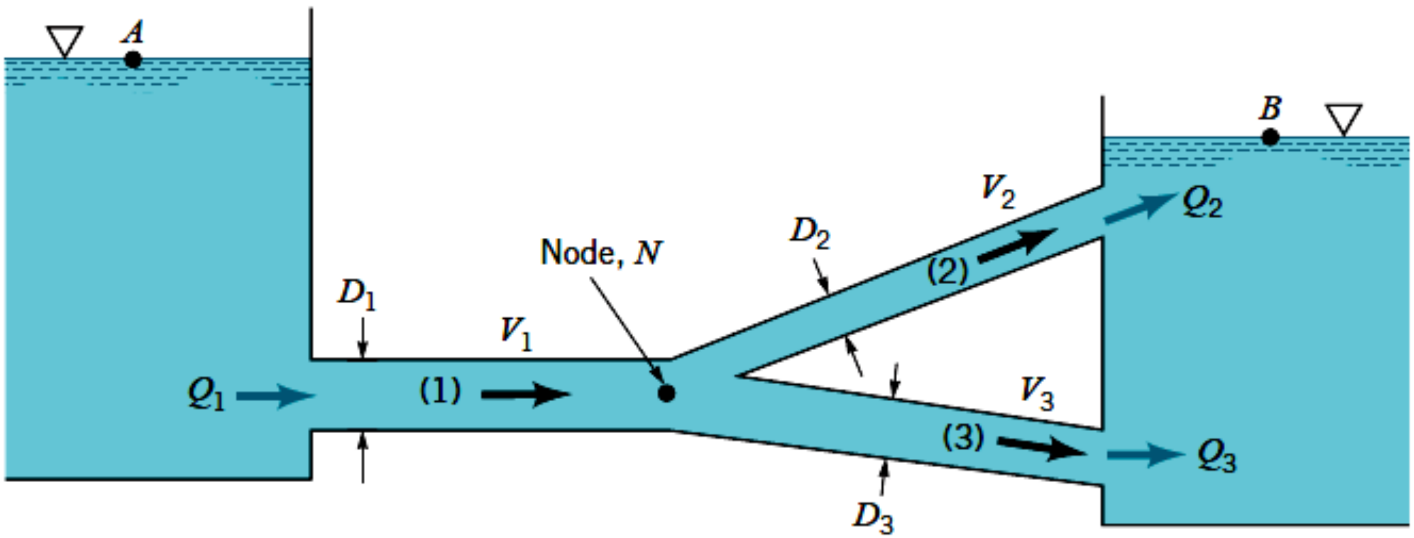
$$\frac{V_1}{V_2} = \left(\frac{f_2}{f_1}\frac{L_2}{L_1}\frac{D_1}{D_2}\right)^{1/2} \quad \text{and} \quad \frac{\dot{V}_1}{\dot{V}_2} = \frac{A_{c,1}V_1}{A_{c,2}V_2} = \frac{D_1^2}{D_2^2}\left(\frac{f_2}{f_1}\frac{L_2}{L_1}\frac{D_1}{D_2}\right)^{1/2}$$

The analysis of piping networks, no matter how complex they are, is based on two simple principles:

1. Conservation of mass throughout the system must be satisfied. This is done by requiring the total flow into a junction to be equal to the total flow out of the junction for all junctions in the system. Also, the flow rate must remain constant in pipes connected in series regardless of the changes in diameters.
2. Pressure drop (and thus head loss) between two junctions must be the same for all paths between the two junctions. This is because pressure is a point function and it cannot have two values at a specified point. In practice this rule is used by requiring that the algebraic sum of head losses in a loop (for all loops) be equal to zero. (A head loss is taken to be positive for flow in the clockwise direction and negative for flow in the counterclockwise direction.)

PIPING NETWORKS

- Another type of multiple pipe system called a ***loop*** is shown in Fig. In this case the flowrate through pipe (1) equals the sum of the flowrates through pipes (2) and (3), or $Q_1 = Q_2 + Q_3$.



- As can be seen by writing the energy equation between the surfaces of each reservoir, the head loss for pipe (2) must equal that for pipe (3), even though the pipe sizes an flowrates may be different for each. That is,

PIPING NETWORKS

$$\frac{p_A}{\gamma} + \frac{V_A^2}{2g} + z_A = \frac{p_B}{\gamma} + \frac{V_B^2}{2g} + z_B + h_{L_1} + h_{L_2}$$

for a fluid particle traveling through pipes (1) and (2), while

$$\frac{p_A}{\gamma} + \frac{V_A^2}{2g} + z_A = \frac{p_B}{\gamma} + \frac{V_B^2}{2g} + z_B + h_{L_1} + h_{L_3}$$

for fluid that travels through pipes (1) and (3). These can be combined to give $h_{L_2} = h_{L_3}$. This is statement of the fact that fluid particles that travel through pipe (2) and particles that travel through pipe (3) all originate from common conditions at the junction (or node, N) of the pipes and all end up at the same final conditions.